DESIGN CRITERIA:

D1. ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARDS.
- A. IBC 2021 BUILDING CODE.
- B. ACI 318 LATEST EDITION
- C. ACI 301 – LATEST EDITION
- D. ACI 530 – LATEST EDITION
- E. ASCE–7–16
- F. AISC LATEST EDITION

D2. DESIGN LOAD VALUES:
- A. SOIL CONTACT PRESSURE < 1000 PSF
- B. SELF WEIGHT.
- C. DEAD LOAD FOR PARTITIONS, MECHANICAL, ELECTRICAL, HVAC = 20 psf.
- D. GROUND SNOW LOAD = 0 psf.
- E. LIVE LOAD SCHEDULE

MWFRS = EXTERIOR SHEARWALLS SEE SHEET

DESIGN LIVE LOAD SCHEDULE
(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

COMPONENT / AREA	RESTURANT		ROOF							
1ST FLOOR	100	–	30	–	–	–	–			
	–	–	–	–	–	–	–			
	–	–	–	–	–	–	–			

D3. DESIGN METHOD ASCE7–16 ENCLOSED SIMPLE DIAPHRAGM

D4. WIND LOAD AS PER ASCE 7–16:
SPEED = 150 MPH (3 SEC. GUST) EXPOSURE – B
IMPORTANCE FACTOR = 1.0
OCCUPANCY CATEGORY = II
INTERNAL PRESSURE COEFFICIENT ALL FLOOR GCpi +/– .18 – ENCLOSED

D5. WINDOWS & DOORS SHALL BE PROTECTED FROM WIND BORNE DEBRIS AND PRESSURES IN SECTION

D5. COMPONENT & CLADDING DESIGN WIND PRESSURES

		+ Pnet 30	– Pnet 30
ROOF ZONE	1	11.3 psf	–38.8 psf
	2	11.3 psf	–50.4 psf
	3	11.3 psf	–63.1 psf
WALL ZONE	4	29.0 psf	–27.7 psf
	5	29.0 psf	–31.8 psf

D6. EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS & NOTES TO ALL OTHER PROJECT DOCUMENTS. SHOULD THERE BE ANY DISCREPANCIES, THE CONTRACTOR SHALL REQUEST A CLARIFICATION IN WRITING.

D7. THE DESIGN OF THIS FOUNDATION IS BASED ON THE
GEOTECHNICAL REPORT BY: SOUTHERN EARTH SCIENCES
PROJECT NO.: B23–169.
DATED: 9–21–23
SITE PREPARATION & DRAINAGE SHALL BE AS PER SOIL REPORT.

GENERAL:

G1. THIS PLAN IS FOR STRUCTURAL REQUIREMENTS ONLY. ARCHITECTURAL DETAILS, SURFACE REQUIREMENTS AND COMPLIANCE WITH A.D.A. REGULATIONS ARE SPECIFICALLY OMITTED FROM THIS PLAN. THE COORDINATION OF, AND RESPONSIBILITY FOR SUCH REQUIREMENTS IS THE RESPONSIBILITY OF OTHERS.

G2. THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.

G3. THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION AND SAFETY.

G4. NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH.

G5. THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.

G6. ALL SUBMITTAL SETS SHALL CONSIST OF 1 ORIGINAL AND 3 COPIES.

G7. ANY SUBMITTALS RECEIVED BY ARCH/ENG THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUBCONTRACTOR SHALL BE RETURNED WITHOUT REVIEW.

G8. ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.

G9. ANY CONFLICTS BETWEEN THE SPECIFICATIONS AND DESIGN DRAWINGS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. THE MORE STRINGENT REQUIREMENTS SHALL GOVERN UNLESS DETERMINED OTHERWISE BY THE ARCHITECT/ENGINEER.

G10. MECHANICAL EQUIPMENT LOADS: THE GENERAL CONTRACTOR SHALL SUBMIT ACTUAL WEIGHTS OF EQUIPMENT TO BE USED IN THE PROJECT TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOADS USED IN THE DESIGN, AND SHALL REPORT ANY CHANGES IN LOCATION, NUMBER OF PEICES, AND WEIGHTS OF EQUIPMENT AS SHOWN ON THE MECHANICAL/ELECTRICAL/PLUMBING DRAWINGS AT LEAST TWO WEEKS PRIOR TO FABRICATION AND CONSTRUCTION OF THE SUPPORTING STRUCTURE.

G11. TYPICAL DETAILS: DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURING ON THE PROJECT THAT ARE THE SAME OR SIMULAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION.

G12. DRAWING CONFLICTS: THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.

G13. ALL STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE STRUCTURE IS TIED TOGETHER AND COMPLETED.

G14. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES AND SEQUENCE.

G15. THE CONTRACTOR SHALL INVESTIGATE THE EXISTING ADJACENT BUILDING, SEWERS AND OTHER UTILITIES AND SHALL TAKE PROPER AND NECESSARY PRECAUTIONS TO PROTECT SAME FROM DAMAGE DUE TO THE EXECUTION OF NEW WORK. SHOULD DAMAGE OCCUR DUE TO THE CONTRACTOR'S NEGLIGENCE, THE COST AND RESPONSIBILITY FOR REPAIRING OR REPLACING THE WORK IN ITS ORIGINAL CONDITION SHALL BE BORNE BY THE CONTRACTOR AT NO COST TO THE OWNER.

G16. SEE OTHER DRAWINGS (HVAC, PLUMBING, ETC.) FOR THE INSTALLATION OF PIPE AND DUCT SLEEVES. THESE G16. SLEEVES SHALL BE STEEL AND SHALL NOT INTERFERE WITH THE STRUCTURAL FRAMING, NOR SHALL THEY IMPAIR THE STRENGTH OF THE STRUCTURE.

FOUNDATION GENERAL NOTES:

1. CONCRETE DESIGN IS BASED UPON A CONCRETE MIX HAVING A MINIMUM OF 5.0/5.2 SACKS OF CEMENT PER CUBIC YARD AND A MAXIMUM OF 30 GALLONS OF FREE AND ADDED WATER PER CUBIC YARD. POUR TO 5±1" SLUMP. SUCH A MIX SHOULD GIVE A MINIMUM COMPRESSION STRENGTH OF 3000 P.S.I. AT 28 DAYS. CONCRETE DESIGN MIX SHALL BE IN ACCORDANCE WITH THE A.C.I. BUILDING CODE REQUIREMENTS (A.C.I. 318R–04).
2. CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1,500 P.S.I. AT THE TIME OF STRESSING.
3. ALL CONVENTIONAL REINFORCING STEEL SHALL BE ASTM DESIGNATION A–615 (GRADE 60) REINFORCING AND SHALL BE DETAILED AND ACCESSORIES PROVIDED IN ACCORDANCE WITH THE LATEST A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES.
4. ALL PRE STRESSING STEEL SHALL CONSIST OF SEVEN–WIRE STRESS RELIEVED STRAND CONFORMING TO ASTM A–416. MINIMUM ULTIMATE TENSILE STRENGTH SHALL BE 270,000 P.S.I. STRANDS SHALL BE COATED WITH A PERMANENT RUST PREVENTATIVE LUBRICANT AND A PLASTIC SHEATH.
5. REINFORCEMENT SHALL HAVE 3" COVER IN GRADE BEAM BOTTOMS, 2" COVER IN BEAM SIDES AND TOPS, 1 1/2" COVER IN SLAB TOPS AND BOTTOMS, UNLESS OTHERWISE SHOWN.
6. TENDONS AND REBARS SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING PLACING OF CONCRETE.
7. THE CONTRACTOR SHALL VERIFY ALL DROPS, SLOPES, RECESSES, BRICK SEATS, BLOCK–OUTS ON ARCHITECTURAL PLANS AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES THAT MAY EXIST.
8. COORDINATE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL OPENINGS, INSERTS, AND ANY OTHER RELATED ITEMS.
9. PLANS FOR PIPES, CONDUITS, THIMBLES, ETC., TO PASS THROUGH CONCRETE SLAB OR BEAM, MUST NOT CONFLICT WITH REINFORCING. WHERE A CONFLICT OCCURS, REINFORCING LOCATION IS TO TAKE PRECEDENCE.
10. PROVIDE (2)–LAYERS OF .006 POLYETHYLENE MEMBRANE UNDER ALL CONCRETE SLABS AND AND GRADE BEAMS.
11. ALL SECTIONS SHOWN ARE THE SECTIONS AT MID–SPAN OF GRADE BEAMS UNLESS OTHERWISE SHOWN.
12. THE TENDON LOCATION AT THE END OF THE GRADE BEAM TO BE A MINIMUM OF 6" FROM THE TOP OF SLAB TO CENTRAL GRAVITY OF TENDONS.
13. TENDONS TO BE STRESSED NO EARLIER THAN 6 DAYS AND NO LATER THAN 14 DAYS AFTER PLACEMENT OF CONCRETE.
14. STRESSING:
    - •• 1/2" TENDON SHALL BE ANCHORED AT 28.9K PER STRAND, BUT SHALL BE INTIALLY STRESSED TO 33.0K PER STRAND.
15. LOADING OF SLAB PRIOR TO TENSIONING SHALL NOT BE DONE WITHOUT THE APPROVAL AND DIRECTION OF THE SUPERVISING ENGINEER.
16. CATHEADS TO BE PLACED ON ALL LIVE ENDS PRIOR TO PLACEMENT OF CONCRETE.
17. THIS DESIGN TO BE USED ONLY FOR THE BELOW LOCATION:

ACADIA PLANTATION RESTAURANT–TRACT A–6A, VILLAGE ONE PROJECT C, THIBODAUX, LA

MECHANICAL FASTENERS:

MF1. ANCHOR RODS FOR STRUCTURAL STEEL SHALL BE WET SET.

MF.2 CARE SHALL BE EXERCISED TO LIMIT THE LENGTH OF FASTENERS TO AVOID DAMAGING TENDONS.

MF3. MASONRY SCREWS SHALL BE "TITEN HD" BY SIMPSON STRONG TIE OR "TAPCON" BY ITW RAMSET/REDHEAD OR ENGINEER–APPROVER EQUAL.

MF4. POWDER–ACTUATED FASTENERS (PAF) SHALL BE BY SIMPSON STRONG TIE, ITW RAMSET/REDHEAD, HILTI, OR ENGINEER–APPROVED EQUAL.

MF5. ALL FASTENERS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS. MF8. ALL HANGERS FOR PLUMBING, MECHANICAL, ELECTRICAL, FIRE PROTECTION SHALL BE CAST INTO SLAB. DRILLING, CORING AND RETRO ATTACHMENTS ARE NOT ALLOWED.

STRUCTURAL STEEL:

SS1. ALL DETAILING, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, THE AWS D1.1 STRUCTURAL WELDING CODE AND MEET THE FOLLOWING REQUIREMENTS:

WIDE FLANGE SHAPES: ASTM A572, YIELD STRENGTH 50 KSI.

CHANNELS, ANGLES, RODS AND PLATES: ASTM A 36, YIELD STRENGTH 36 KSI EXCEPT WHERE NOTED OTHERWISE IN SECTIONS.

TUBE / HSS: ASTM A500 GRADE B.
PIPE: ASTM A53 GRADE B.
HIGH STRENGTH BOLTS: ASTM A325N
ANCHOR BOLTS: ASTM A307.
WELDING MATERIALS: E70XX.
PAINT: APPROVED PRIMER – 2 MILS THICK. DO NOT PAINT SURFACES TO BE WELDED, EMBEDDED IN CONCRETE OR MASONRY, OR CONTACT SURFACES OF FRICTION CONNECTIONS.

SS2. ALL WELDING BY CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1–94, STRUCTURAL WELDING CODE. ALL ELECTRODES USED FOR SUBMERGED ARC AND SHIELDED METAL ARC WELDING SHALL BE COMPATIBLE WITH THE STRUCTURAL STEEL AS SPECIFIED IN AWS AND AISC. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND SITE DRAWINGS FOR DIMENSIONS AND DETAILS REQUIRED FOR STRUCTURAL STEEL WORK NOT SHOWN ON THE STRUCTURAL DRAWINGS.

SS3. ALL STRUCTURAL STEEL EXPOSED TO THE EXTERIOR OF THE BUILDING SHALL BE HOT DIPPED GALVANIZED.

SS12. PROVIDE GROUT FOR BASE PLATES THAT IS NON–SHRINK, NON–METALLIC GROUT WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6000 PSI. COMPLETE GROUT WORK PRIOR TO PLACING ROOF CONCRETE OF A SINGLE STORY BUILDING OR PRIOR TO PLACING SECOND FLOOR CONCRETE OF A MULTIPLE STORY BUILDING.

SS14. PROVIDE WASHERS FOR ALL CONNECTIONS WITH STANDARD, OVERSIZE AND SHORT–SLOTTED HOLES; FOR LONG–SLOTTED HOLES PROVIDE WASHERS OR A CONTINUOUS BAR OF SUFFICIENT SIZE TO COMPLETELY COVER THE SPOT. PLATE WASHERS OR BARS TO BE MINIMUM OF 5/16 INCH THICK FOR LONG–SLOTTED HOLES.

SS15. WIDE FLANGE BEAM CONNECTION TO TUBE COLUMNS SHALL BE MADE WITH BOLTED SHEAR TAB PLATE TYPE CONNECTIONS. ONE–SIDED CONNECTIONS SHALL BE DESIGNED AS ECCENTRIC CONNECTIONS.

SS16. FURNISH STEEL SHOP DRAWINGS FOR ARCHITECT'S AND STRUCTURAL ENGINEER'S REVIEW PRIOR TO FABRICATION. INCLUDE WELDING PROCEDURES, TESTING PROGRAMS FOR WELDING AND HIGH STRENGTH BOLTING, COATING MATERIAL AND ERECTION SEQUENCE ON SHOP DRAWINGS. SHOP DRAWINGS SHALL NOT BE REPRODUCTIONS OF CONTRACT DOCUMENT.

LIGHT GAGE METAL FRAMING:

LMF1. CONTRACTOR SHALL COORDINATE TESTING AGENCY AND /OR INSPECTIONS AS PER PROJECT PLANS AND SPECIFICATIONS. SUBMIT REPORTS AS PER CODE OR SPECIFICATIONS.

LMF2. COLD–ROLLED EXTERIOR WALLS ARE DESIGNED BY ENGINEER OF RECORD. ALTERNATIVE ENGINEERED DESIGNS MAY BE PROPOSED.

LMF3. FOR INTERIOR NON STRUCTURAL WALLS, CONNECT TRACKS TO CONCRETE "POWER ACTUATED FASTENERS" (PAF) WITH A DIAMETER OF 0.145" AND A MAX PENETRATION OF 1–1/4" AT 32" OC EACH STUD SHALL BE CONNECTED TO THE TRACK W/ A MIN OF 2 #18 SCREWS AT EACH END. DRAWINGS FOR EXACT LOCATION AND EXTENTS OF STEEL STUD WORK.

FOR ALL WALLS THE TRACK SHALL BE OF EQUAL WIDTH AND GAGE TO THE STUD MATERIAL U.N.O.

POWER ACTUATED FASTENERS SHALL HAVE THE FOLLOWING MINIMUM SAFE WORKING CAPACITIES
IN 3000 PSI CONCRETE w/ 1" EMBEDMENT 70# TENSION 145# SHEAR
IN 3000 PSI CONCRETE w/ 1.25" EMBEDMENT 195# TENSION 265# SHEAR
IN STRUCTURAL STEEL (MIN 3/16 " THICKNESS) 155# TENSION 395# SHEAR

LMF4. EXTERIOR STRUCTURAL (LOAD BEARING) STEEL STUDS SHALL MEET FOLLOWING DESIGN CRITERIA: GALV G–60 ASTM A525, MINIMUM YIELD STRESS SHALL BE 50,000 PSI FOR 16ga/54m. CONNECT TRACKS TO CONCRETE W/ ANCHOR BOLTS AND P.A.F. PER DETAIL "1" ON SHEET S–8. EACH STUD SHALL BE CONNECTED TO THE TRACK W/ A MIN OF 2 #10 SCREWS AT EACH END DRAWINGS FOR EXACT LOCATION AND EXTENTS OF STEEL STUD WORK.

LMF 5. STUD AND TRACK SIZES ARE AS FOLLOWS:

EXTERIOR STRUCTURAL STUDS

	WIDTH	FLANGE	GAGE	AREA	Ix	Sx	Rx	NOMENCLATURE
C STUD	6"	2"	16/54m	.613	3.32	1.11	2.33	600S200–54
TRACK	6"	2"	16/54m	.566	3.147	1.016	2.359	600T200–54

INTERIOR NON–STRUCTURAL STUDS

	WIDTH	FLANGE	GAGE	AREA	Ix	Sx	Rx	NOMENCLATURE
C STUD	3–5/8"	1–5/8"	16/54m	.422	0.873	0.482	1.44	362S162–54
C STUD	6"	1–5/8"	16/54m	.613	2.86	0.954	2.27	600S162–54
TRACK	3–5/8"	2"	16/54m	.431	1.025	1.542	1.542	362T200–54
TRACK	6"	2"	16/54m	.566	3.147	1.016	2.359	600T200–54

ENTRY CANOPY STUDS (RAFTERS)

	WIDTH	FLANGE	GAGE	AREA	Ix	Sx	Rx	NOMENCLATURE
C JOIST	8"	1–5/8"	16/54m	.670	5.74	1.435	2.927	800S162–54

NOTES: NOT ALL SIZES MAY BE USED. SEE PLANS AND SECTIONS FOR SIZE(S) REQUIRED.
STRUCTURAL DRAWINGS SHALL GOVERN OVER ARCHITECTURAL DRAWINGS.

FOR NON STRUCTURAL PARTITION WALLS REFER TO ARCHITECTURAL DRAWINGS.

LMF5. EXECUTION:

POSITION STUDS VERTICALLY IN TRACKS AND SPACE AS PER SCHEDULE. SECURELY ANCHOR EACH STUD TO TRACKS WITH MIN. #10 SCREWS, TWO AT TOP AND TWO AT BOTTOM, WITH ONE SCREW IN EACH FLANGE. TRACKS SHALL BE SECURELY ANCHORED TO THE SUPPORTING STRUCTURE AS SHOWN ON THE PLANS. AT TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, OR THEY SHALL BE BUTT–WELDED OR SPLICED TOGETHER. TRIMMER STUDS OR CRIPPLES SHALL BE INSTALLED BELOW WINDOW SILLS, BELOW WINDOW AND DOOR HEADERS, AT FREE STANDINGS STAIR RAILS, AND ELSEWHERE TO FURNISH SUPPORT, AND SHALL BE SECURELY ATTACHED TO SUPPORTING MEMBERS. PROVISIONS FOR STRUCTURE VERTICAL MOVEMENT SHALL BE PROVIDED WHERE INDICATED ON THE PLANS. SPLICES IN AXIALLY LOADED STUDS SHALL NOT BE PERMITTED.

LMF6. EXTERIOR SHEATHING:

EXTERIOR WALLS – 5/8" DENSE GLASS. ATTACHED WITH #10 x 1" WAFER HEAD: AT CORNERS (8'x12') 4" EDGE, 6" FIELD BETWEEN CORNERS @ 6" EDGE, 12" FIELD.
OR PER SHEATHING MANUFACTURER.

LMF7. LATERAL BRACING (HORIZONTAL BRIDGING) FOR WALLS:

NON–LOAD–BEARING INTERIOR WALLS (PARTITION WALLS) HAVE SHEATHING ON BOTH SIDES, LATERAL BRACING IS ONLY REQUIRED AT MID HEIGHT. SEE DETAILS ON SHEET S–7

LOAD–BEARING WALLS (STRUCTURAL WALLS) ENSURE BOTH STUD FLANGES ARE ATTACHED TO TOP AND BOTTOM TRACK FLANGES. HOROZONAL BRIDGING SHALL BE INSTALLED AT SPACING NOT TO EXCEED 48" OC VERTICALLY 1 1/2" COLD ROLLED CHANNELS MAY BE USED TO BRACE STUDS LATERALLY. CHANNELS ARE INSERTED THROUGH STUD WEB HOLES AND SECURED WITH SCREW–ATTACHED 1 1/2"x2"x16ga CLIP ANGLES CUT TO LENGTH 1/4" LESS THAN STUD WIDTH.

LMF8. PROVIDE A VERTICAL 3" x 14ga CONTINUOUS SPLICE PLATE BETWEEN TRACK AND STUDS FOR RADIUS WALLS.

LMF12. CONTRACTOR SHALL COORDINATE ALL NECESSARY BLOCKING / BRIDGING FOR WALL MOUNTED ITEMS (CABINETS, GRAB BARS, URINALS, ECT.)

METAL ROOF DECK- 1.5B20:

MRD1. STEEL DECK SHALL MEET THE REQUIREMENTS OF ASTM A 653, GRADE 33 AND GALVANIZED MEETING THE REQUIREMENTS OF ASTM A 653 (CLASS G90) PROVIDE DECK COMPLYING WITH SDI "ROOF DECK SPECIFICATIONS", 20ga. TYPE B, 1–1/2" DEEP, (6"x2–1/2"x1–1/2") W/ 36" COVERAGE.

THE METAL ROOF DECK SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

THICKNESS	=	.0358 in
Sp	=	.234 in³
Sn	=	.247 in³
Ip	=	.201 in⁴
In	=	.222 in⁴
WEIGHT	=	2.14 psf

MRD2. FASTEN DECK TO SUPPORTING MEMBERS @ :

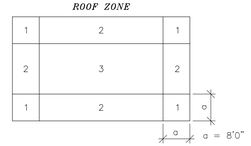
ZONE	SPACING	SCREW
1	36/7 @ 24" O.C.	#12
2	36/7 @ 24" O.C.	#12
3	36/7 @ 48" O.C.	#12

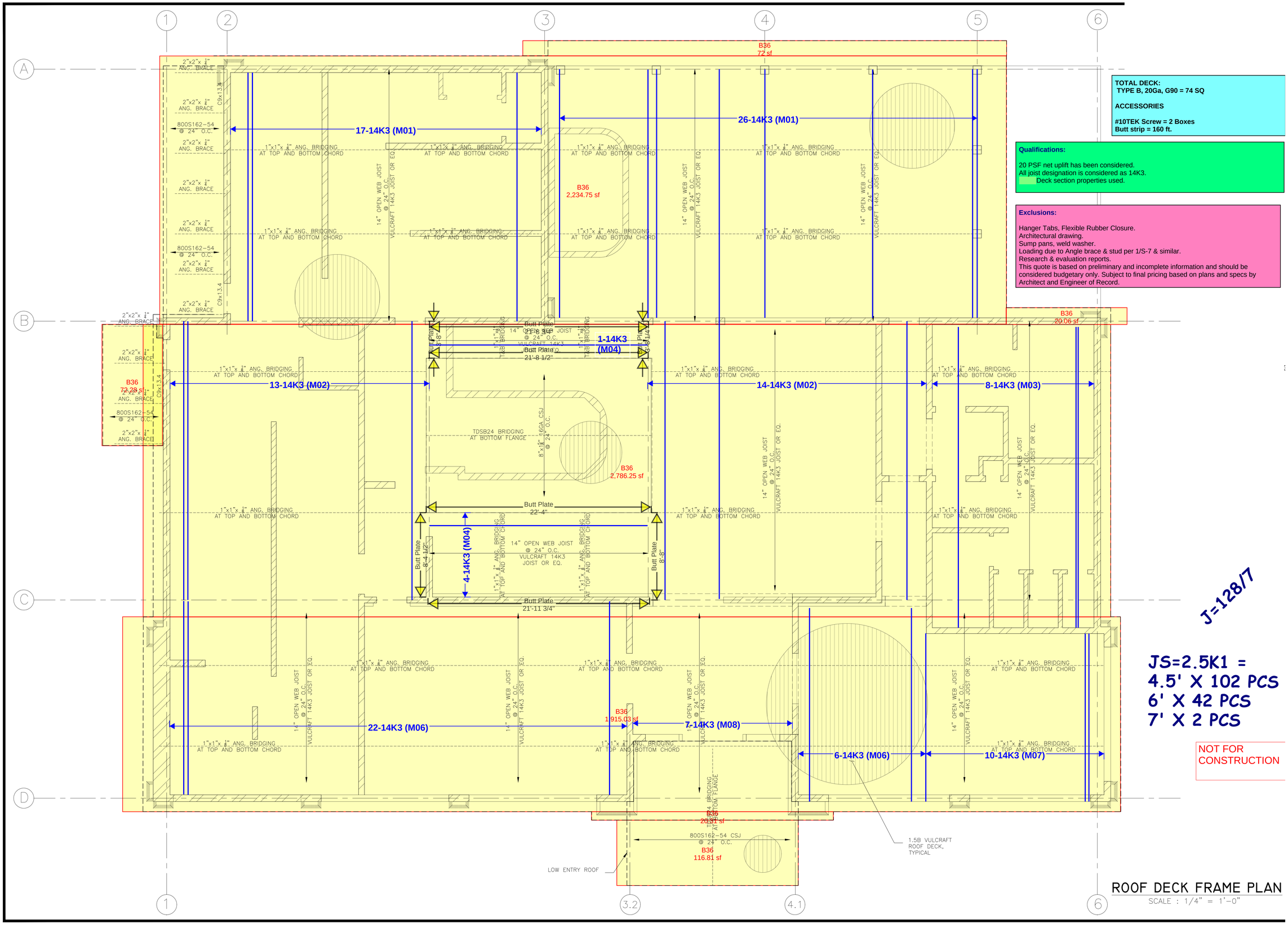
NOT FOR CONSTRUCTION

MRD3. FASTEN DECK ALONG STITCH OF ADJACENT DECK UNITS WITH #10 TEK SCREWS @ 24" O.C.

MRD4. #10 SCREWS .190 SHANK / .415 – .400 HEAD DIAMETER.
#12 SCREWS .210 SHANK / .43 – .400 HEAD DIAMETER.

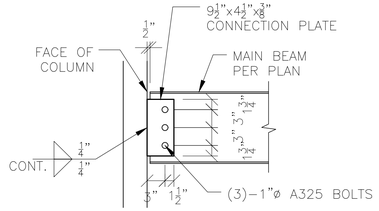
MRD5.





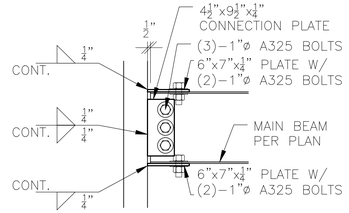
ROOF DECK FRAME PLAN

SCALE : 1/4" = 1'-0"



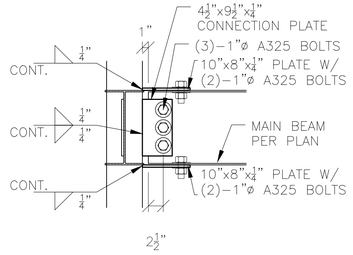
A) BEAM TO COLUMN PINNED CONNECTION

SCALE: 1" = 1'-0"



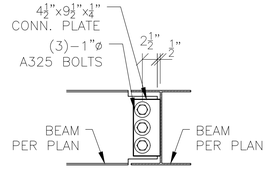
B) BEAM TO COLUMN MOMENT CONNECTION

SCALE: 1" = 1'-0"



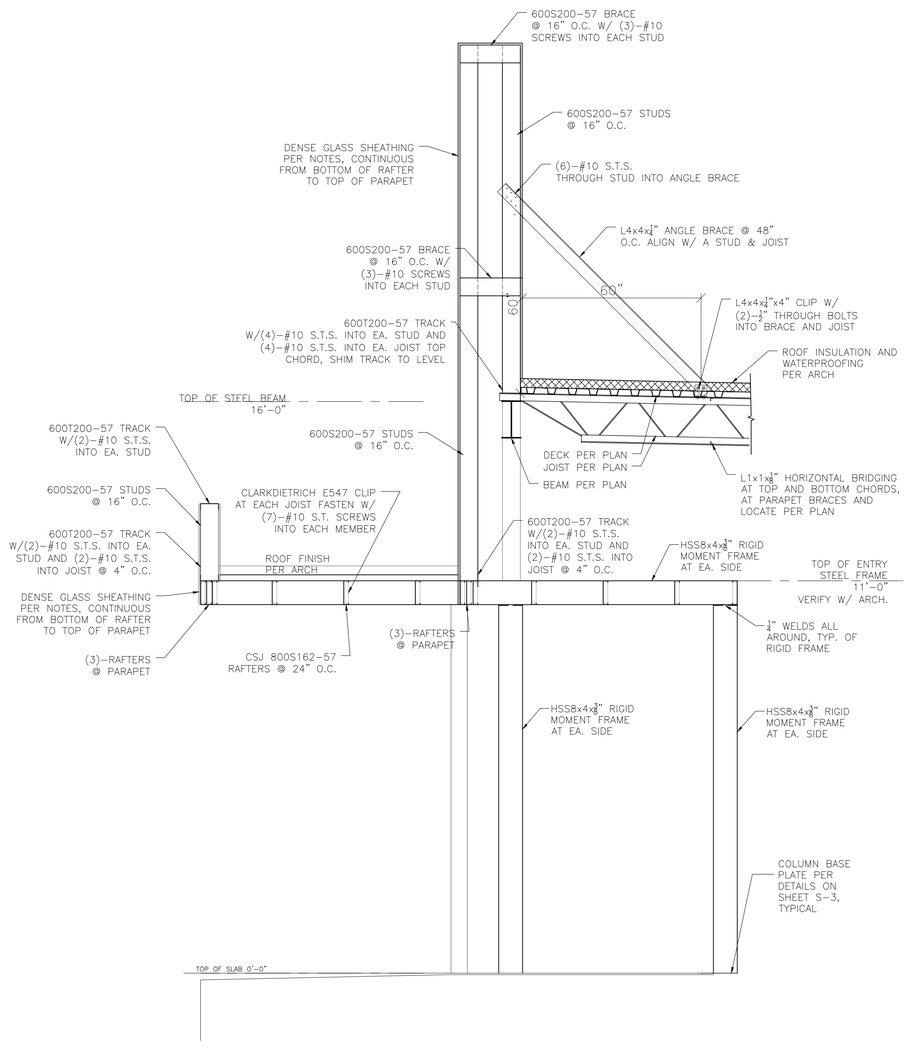
C) BEAM TO COLUMN

SCALE: 1" = 1'-0"



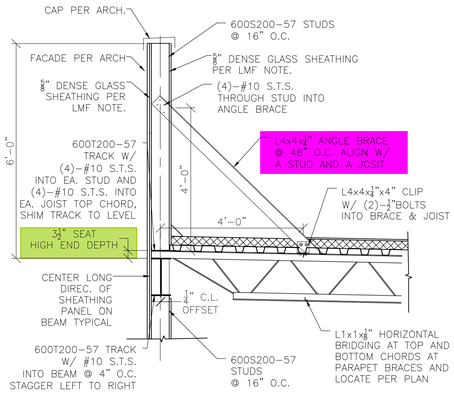
D) BEAM TO BEAM

SCALE: 1" = 1'-0"



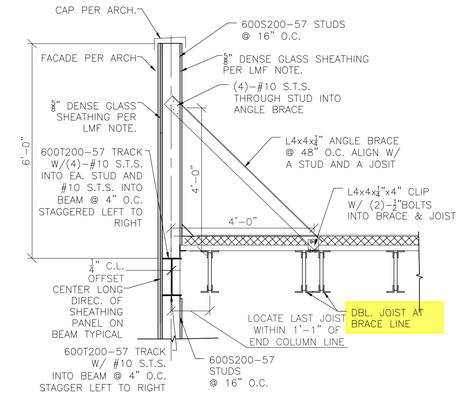
SECTION 1 - FRONT ENTRY

SCALE: 1/2" = 1'-0"



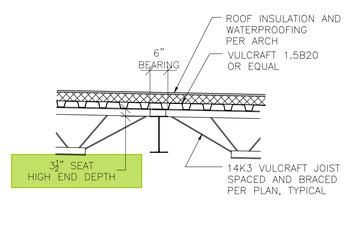
SECTION 2 - PARAPET PERP.

SCALE: 1/2" = 1'-0"



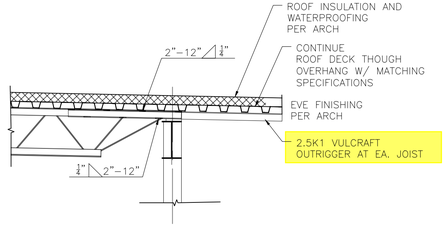
SECTION 3 - PARAPET PARALLEL

SCALE: 1/2" = 1'-0"



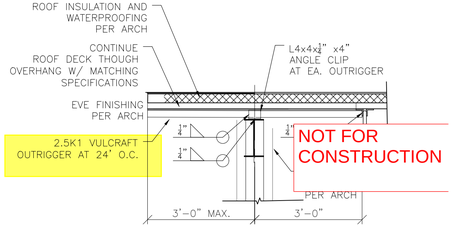
SECTION 4 - INTERIOR JOIST BEARING

SCALE: 1/2" = 1'-0"



SECTION 5 - JOIST EXTENSION PARALLEL

SCALE: 1/2" = 1'-0"



SECTION 6 - JOIST EXTENSION PERP.

SCALE: 1/2" = 1'-0"

JOISTS

NOTES :

LL DEFL **L/240** TL DEFL JOIST SPACING

DWG	MARK	QTY	BCX	DESIGNATION	SPAN	SPACE	NU PSF	TCX L	TCX R	COMMENTS
	M01	43	6	14K3	25'-0"	2'	20			3.5" joist seat depth both end
	M02	27	1	14K3	27'-7 5/8"	2'	20			3.5" joist seat depth both end
	M03	8		14K3	31'-0"	2'	20			3.5" joist seat depth both end
	M04	5		14K3	21'-9"	2'	20			3.5" joist seat depth both end
	M06	28		14K3	19'-7 5/8"	2'	20			3.5" joist seat depth both end
	M07	10		14K3	16'-7 1/4"	2'	20			3.5" joist seat depth both end
	M08	7		14K3	14'-0"	2'	20			3.5" joist seat depth both end
	TOTAL	128	7							