GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.

2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.

3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.

4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.

5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.

6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.

7. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.

8. CONTRACTOR SHALL VERIFY THE STRUCTURALLY SUPPORTED MECHANICAL EQUIPMENT WEIGHTS, OPENING SIZES AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

9. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS.

10. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

11. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.

12. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.

13. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.

14. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

15. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.

16. STRUCTURAL DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR THE DESIGN OF CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.

17. SUBMITTALS

    17.1 SUBMITTALS BY THE CONTRACTOR ARE NOT A PART OF THE CONTRACT DOCUMENTS. PRIOR TO THE INITIAL SUBMITTAL, CONTRACTOR SHALL SUBMIT TO THE DESIGN PROFESSIONAL A SCHEDULE OF SUBMITTED INFORMATION.

    17.2 SUBMITTALS SHALL BE ACCOMPANIED BY A TRANSMITTAL LETTER WITH THE FOLLOWING INFORMATION:

    - PROJECT NAME
    - CONTRACTOR'S NAME
    - DATE SUBMITTED
    - DESCRIPTION OF ITEMS SUBMITTED. IDENTIFY WORK AND PRODUCT BY SPECIFICATION SECTION
    - NUMBER OF DRAWINGS AND OTHER PERTINENT DATA.

    17.3 CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION ON THE SUBMITTAL TO ANY DEVIATION FROM THE CONTRACT DOCUMENTS. CONTRACTOR SHALL STAMP AND SIGN EACH SHEET OF SHOP DRAWINGS AND PRODUCT DATA, AND SIGN OR INITIAL EACH SAMPLE TO CERTIFY COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS. SUBMITTALS RECEIVED WITHOUT THE CONTRACTOR'S STAMP OF REVIEW WILL BE RETURNED TO THE CONTRACTOR FOR REVIEW AND RESUBMITTAL.

    17.4 WORK REQUIRING SHOP DRAWINGS, WHETHER CALLED FOR BY THE CONTRACT DOCUMENTS OR REQUESTED BY THE CONTRACTOR, SHALL NOT COMMENCE UNTIL THE SUBMISSION HAS BEEN REVIEWED BY THE DESIGN PROFESSIONAL. WORK MAY COMMENCE IF THE CONTRACTOR VERIFIES THE ACCURACY OF THE DESIGN PROFESSIONAL'S CORRECTIONS AND NOTATIONS AND COMPLIES WITH THEM WITHOUT EXCEPTION AND WITHOUT REQUESTING CHANGE IN CONTRACT SUM OR CONTRACT TIME AT COPY OF THE MARKED STRUCTURAL SHOP DRAWINGS WITH THE DESIGN PROFESSIONAL'S REVIEW STAMP IS TO BE MAINTAINED AT THE JOB SITE.

CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
   - INTERNATIONAL BUILDING CODE, 2018 EDITION WITH GEORGIA AMENDMENTS.

2. GRAVITY LOADS

   2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

CLASSROOMS	40	PSF
OFFICES	50	PSF
CORRIDORS	100	PSF
LIBRARY READING ROOM	60	PSF
LIBRARY STACK	150	PSF
GYMNASIUM	100	PSF
STAGE FLOOR	150	PSF
CAFETERIA	100	PSF
MECHANICAL ROOMS	125	PSF
STAIRS	100	PSF
STORAGE	125	PSF
PARTITIONS	15	PSF

   2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

ROOF, L_r	20	PSF
GROUND SNOW LOAD, P_g	0	PSF
RAIN LOAD, R	20	PSF

   PONDING AND DRIFT EFFECTS HAVE BEEN INCLUDED IN THE DESIGN.

   2.3 CONCENTRATED FLOOR LOADS: DISTRIBUTED OVER AN AREA OF 2-1/2 FEET BY 2-1/2 FEET, UNLESS NOTED OTHERWISE:

OFFICE	2000	LB
SCHOOLS	1000	LB
LIBRARY READING ROOM	1000	LB
LIBRARY STACK ROOM	1500	LB

   2.4 DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):

   ROOF:

ROOFING	3	PSF
INSULATION	6	PSF
MISCELLANEOUS	3	PSF
CEILING/MEP	6	PSF

3. WIND LOADS:

   CLASSROOM WINGS, COMMONS AND CAFETERIA, AND GYM:
   - BASIC DESIGN WIND SPEED, V = 121 MPH
   - ALLOWABLE DESIGN WIND SPEED, V_{ASD} = 93.7 MPH
   - RISK CATEGORY: III
   - EXPOSURE C
   - INTERNAL PRESSURE COEFFICIENT = +/- 0.18
   - DESIGN BASE SHEAR:
     CLASSROOM WINGS: V_x = 21.5 KIPS, V_y = 60.3 KIPS
     CORE AREA: V_x = 135 KIPS, V_y = 160 KIPS
     CAFETERIA/GYM: V_x = 150 KIPS, V_y = 205 KIPS

   BUS CANOPY:
   - BASIC DESIGN WIND SPEED, V = 113 MPH
   - ALLOWABLE DESIGN WIND SPEED, V_{ASD} = 87.0 MPH
   - RISK CATEGORY: II
   - EXPOSURE C
   - INTERNAL PRESSURE COEFFICIENT = +/- 0.00
   - DESIGN BASE SHEAR: V_x = 3 KIPS, V_y = 20 KIPS

   SEE COMPONENT AND CLADDING DESIGN WIND PRESSURE DIAGRAM

4. EARTHQUAKE LOADS:

   CLASSROOM WINGS, COMMONS AND CAFETERIA, AND GYM:
   - RISK CATEGORY: III
   - SEISMIC IMPORTANCE FACTOR: I = 1.25
   - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_S = 0.154
   - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_1 = 0.073
   - SITE CLASS D
   - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{DS} = 0.164
   - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{D1} = 0.117
   - SEISMIC DESIGN CATEGORY: B
   - BASIC SEISMIC-FORCE RESISTING SYSTEM: INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
   - DESIGN BASE SHEAR:
     CLASSROOM WINGS: V = 41.4 KIPS
     CORE AREA: V = 120 KIPS
     CAFETERIA/GYM: V = 102 KIPS
   - SEISMIC RESPONSE COEFFICIENT, C_S = 0.059
   - RESPONSE MODIFICATION FACTOR, R = 3.5
   - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

   BUS CANOPY:
   - RISK CATEGORY: II
   - SEISMIC IMPORTANCE FACTOR: I = 1.0
   - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_S = 0.154
   - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_1 = 0.073
   - SITE CLASS D (ASSUMED)
   - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{DS} = 0.164
   - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, S_{D1} = 0.117
   - SEISMIC DESIGN CATEGORY: B
   - BASIC SEISMIC-FORCE RESISTING SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
   - DESIGN BASE SHEAR: V = 3 KIPS

5. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:

	DEAD LOAD	LIVE LOAD	DEAD + LIVE LOAD
ROOF MEMBERS:	L/360	L/360	L/240

   - WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS. (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.
   - THE CALCULATED DEFLECTION FOR INDIVIDUAL MEMBERS SUPPORTING MASONRY DO NOT EXCEED L/600 FOR DESIGN LOADS APPLIED AFTER THE INSTALLATION OF THE MASONRY.

6. SPECIAL INSPECTIONS:

   6.1 THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SPECIFICATION SECTION 014525, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. MATERIALS AND WORK TO BE INSPECTED INCLUDE CONCRETE, STEEL, AND MASONRY CONSTRUCTION. SEE SPECIFICATION SECTIONS 014525 FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.

   6.2 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO FLOOR AND ROOF TRUSSES AND JOISTS OF WOOD AND STEEL MATERIALS, STRUCTURAL STEEL FRAMING, AND PRECAST CONCRETE, JOISTS, BEAMS, COLUMNS, SLABS, WALLS AND CLADDING.

   6.3 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

   6.4 THE PROJECT OWNER WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

   6.5 APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF 8 COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE AND TO THE DESIGN PROFESSIONAL WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY THE CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.

   6.6 SPECIAL INSPECTION REPORTS AND FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.

7. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY NOVA ENGINEERING AND ENVIRONMENTAL, REPORT NUMBER 10102-2024048 DATED APRIL 8, 2024. DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.

2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.

3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 2500 PSF .

   3.1 NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.

4. FOUNDATION WALLS ARE DESIGNED FOR LATERAL PRESSURES DUE TO THE FOLLOWING ASSUMED EQUIVALENT FLUID DENSITIES:

WALLS SUPPORTED AT TOP (AT-REST CONDITION):	60	PCF
WALLS FREE TO DISPLACE AT TOP (ACTIVE CONDITION):	45	PCF
PASSIVE PRESSURE	240	PCF
SLIDING COEFFICIENT OF FRICTION	0.35	

5. BACKFILL PLACED AGAINST EXTERIOR OR RETAINING WALLS SHALL NOT EXCEED 120 PCF WEIGHT FOR WET UNIT WEIGHT OF SOIL.

6. PROOF ROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP-TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.

7. DENSIFY BUILDING AREAS AND A MINIMUM OF 15'-0" OUTSIDE THE BUILDING PERIMETER USING A VIBRATORY ROLLER (SEE SPECIFICATIONS).

8. STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL FILL UNDER SLABS AND WITHIN 10'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN LIFTS OF THICKNESS DETERMINED BY THE INDEPENDENT TESTING AGENCY AND COMPACTED TO AT LEAST 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698. THE TOP 12" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL, COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY. STRUCTURAL FILL SOIL DENSITY SHALL BE AT LEAST 95 PCF.

9. SLABS-ON-GRADE SHALL BE PLACED ON A 6" GRANULAR BASE, COMPACTED TO 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698, AND COVERED WITH A CONTINUOUSLY SEALED VAPOR BARRIER. SEE ARCHITECT FOR THICKNESS OF VAPOR BARRIER THE BASE FOR SLABS-ON-GRADE SHALL BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO EACH PLACEMENT OF CONCRETE.

10. BACKFILL SHALL NOT BE PLACED AGAINST EXTERIOR OR RETAINING WALLS UNTIL THE WALLS HAVE ACHIEVED THEIR DESIGN STRENGTH AND THEIR LATERAL SUPPORT ELEMENTS ARE INSTALLED. PROVIDE ADEQUATE DRAINAGE AT BASEMENT AND RETAINING WALLS (SEE ARCHITECTURAL).

11. FOOTINGS SHALL BE CENTERED ABOUT COLUMN LINES UNLESS NOTED OTHERWISE.

12. ALL FOOTINGS AND TURN DOWN SLAB EDGES SHALL PENETRATE TO A MINIMUM DEPTH OF 12" BELOW FINISHED GRADE.

13. ALL TEMPORARY GROUNDWATER CONTROL TO BE PROVIDED BY CONTRACTOR. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING AND COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.

2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAPS OF 8".

3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.

4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.

5. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.

6. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

   6.1 CONCRETE REINFORCEMENT COVER

   EXPOSED TO EARTH OR WEATHER:

UNFORMED CAST AGAINST EARTH	3" CLEAR
FORMED #6 AND LARGER	2" CLEAR
FORMED #5 AND SMALLER	1-1/2" CLEAR

   NOT EXPOSED TO EARTH OR WEATHER:

SLABS	3/4" CLEAR

   6.2 MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF THE WALL UNLESS NOTED OTHERWISE.

7. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:
   - CONCRETE REINFORCEMENT: CLASS B TENSION LAP
   - MASONRY REINFORCEMENT: SEE DETAIL 1/S4.01

8. ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL CONFORM TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. REINFORCING INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.

10. ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.

11. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS, UNLESS SHOWN OTHERWISE IN DETAILS.

CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.

2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:

   2.1 NORMAL WEIGHT STRUCTURAL CONCRETE:

FOOTINGS	3000 PSI	F0
INTERIOR SLABS-ON-GRADE	4000 PSI	F0
EXTERIOR SLABS-ON-GRADE	4500 PSI	F2
PIERS	4500 PSI	F2
WALLS	4500 PSI	F2

3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.

4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.

5. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.

6. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.

CONCRETE MASONRY

1. MINIMUM 28-DAY COMPRESSIVE STRENGTH OF ASTM C90 CONCRETE MASONRY UNITS SHALL BE F'M = 2000 PSI.

2. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY = 2000 PSI.

3. MORTAR SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND ASTM C270. MORTAR SHALL BE OF THE FOLLOWING TYPE:
   - WALLS BELOW GRADE: TYPE M
   - BEARING WALLS: TYPE M OR S

4. CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO ASTM C476.

5. PROVIDE HORIZONTAL LADDER-TYPE JOINT REINFORCEMENT WITH NO. 9 GAGE DEFORMED LONGITUDINAL WIRES AT 16" C/C VERTICALLY AND AT 8" BELOW GRADE, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC. LONGITUDINAL WIRES SHALL BE PLACED IN THE MORTAR JOINTS.

6. PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.

7. PROVIDE CONTROL JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE DESIGN PROFESSIONAL AT A MAXIMUM SPACING OF 3 TIMES THE WALL HEIGHT OR 25'-0", WHICHEVER IS LESS.

8. PROVIDE DOVETAIL ANCHORS AT 16" C/C, UNLESS NOTED OTHERWISE, WHERE MASONRY WALLS ABUT CONCRETE SURFACES.

9. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.

10. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #4@48" CENTERED, UNLESS NOTED OTHERWISE.

11. MINIMUM VERTICAL WALL REINFORCEMENT FOR INTERIOR NON-LOAD BEARING PARTITION WALLS SHALL BE #4@48" CENTERED, UNLESS NOTED OTHERWISE.

12. SUBMIT SHOP DRAWINGS FOR MASONRY REINFORCEMENT IN ACCORDANCE WITH SPECIFICATION SECTION 032000.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
   - STRUCTURAL STEEL HSS SHAPES SHALL CONFORM TO ASTM A500, GRADE C.
   - MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.

2. BOLTS AND ANCHORS:

   2.1 BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE) WITH MINIMUM 3/4" DIAMETER F3125 BOLTS. SUBMIT PROPOSED BOLT TIGHTENING PROCEDURE FOR REVIEW. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2014 (SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS).

   2.2 ANCHOR BOLTS SHALL BE HEADED BOLTS CONFORMING TO ASTM F1554 GRADE 36 AND SHALL BE HEADED RODS OR THREADED RODS WITH HEAVY HEXAGONAL NUT WELDED TO THE BOTTOM OF THE THREADED ROD, GRADE A563A, UNLESS NOTED OTHERWISE.

   2.3 EXPANSION ANCHORS SHALL HAVE BEEN EVALUATED BY THE ICC EVALUATION SERVICES, INC (ICC-ES) WITH A PUBLISHED EVALUATION REPORT. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 193 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. ALL ANCHORS SHALL BE APPROVED FOR RESISTING WIND AND SEISMIC LOADS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.

   2.4 ADHESIVE ANCHORS SHALL CONSIST OF AN ALL-THREAD STEEL ANCHOR WITH ADHESIVE CONFORMING TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.

3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRAC DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION VALUES SHOWN ON THE STRUCTURAL DRAWINGS. FOR STEEL MEMBERS WHOSE REACTIONS ARE NOT SHOWN, THE DESIGN REACTION SHALL BE AS LISTED IN THE PLAN NOTES. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE DESIGN PROFESSIONAL. **DESIGN PROFESSIONAL SHALL BE COMPENSATED BY THE CONTRACTOR FOR THE COST INVOLVED IN THE REDESIGN OF CONNECTIONS FOR THE CONVENIENCE OF THE CONTRACTOR. STEEL CONNECTIONS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS FOR SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.** REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. FOR CONNECTION DETAILS DEPICTING ARRANGEMENT CONCEPT OF THE CONNECTION WITHOUT COMPLETE DETAILS, THE CONNECTION DESIGN ENGINEER SHALL FOLLOW THAT ARRANGEMENT CONCEPT IN THE DESIGN. SINGLE ANGLE CONNECTIONS ARE NOT ACCEPTABLE.

5. USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY D1.1/D1.1M-2015. "NON-PRE-QUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

6. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED.

STEEL JOISTS

1. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED, FABRICATED, AND ERECTED ACCORDING TO THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).

2. STEEL ROOF JOISTS AND BRIDGING SHALL BE DESIGNED FOR A NET UNIFORM UPLIFT LOAD AS INDICATED ON THE COMPONENTS AND CLADDING DIAGRAM MINUS THE SELF WEIGHT OF THE JOIST AND 5 PSF SUPERIMPOSED DEAD LOAD.

3. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR ROOF JOISTS NOT BRACED BY STEEL ROOF DECK.

4. DESIGN OF STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE STEEL JOISTS, BRIDGING AND THEIR CONNECTIONS. SUBMIT CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.

5. CONTRACTOR SHALL COORDINATE THE CONSTRUCTION AND ERECTION OF WALLS, BEAM FRAMING, METAL DECKING, ETC. TO ENSURE COMPATIBILITY OF ROOF AND WALL SYSTEMS CONSIDERING PITCH AND CAMBER OF STEEL JOISTS.

6. ALL JOIST SEATS SHALL BE CAPABLE OF RESISTING A 2,000 LB SERVICE OVERTURNING FORCE, UNO.

7. JOISTS SHALL BE CAMBERED ACCORDING TO THE SJI SPECIFICATIONS.

METAL DECK

1. DECK DESIGN IS BASED ON THE STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS.

2. PROVIDE GALVANIZED ROOF DECK WITH THE FOLLOWING MINIMUM PROPERTIES:

1 1/2 INCH	DEPTH
20 GAGE	THICKNESS
0.234 IN^3/FT	SECTION MODULUS
0.201 IN^4/FT	MOMENT OF INERTIA
33,000 PSI	YIELD STRESS

3. DECK IS SPECIFIED BASED ON A THREE SPAN CONDITION. FURNISH HEAVIER GAGE DECK IF REQUIRED FOR ONE OR TWO SPAN CONDITIONS.

WOOD

1. PLYWOOD DIAPHRAGMS SHALL BE APA RATED SHEATHING, SOUTHERN PINE PLYWOOD WITH THICKNESS AS NOTED IN THE STRUCTURAL DOCUMENTS. PLYWOOD SHALL CONFORM TO THE REQUIREMENTS OF THE BUILDING CODE.

2. PLYWOOD SHALL BE ORIENTED AND NAILED TO SUPPORTING MEMBERS AS NOTED IN THE STRUCTURAL DOCUMENTS.

3. PLYWOOD SHALL BE PROVIDED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN PLYWOOD ASSOCIATION (APA). THE MINIMUM THICKNESSES WHICH FOLLOW SHALL BE INCREASED AS REQUIRED TO SATISFY ARCHITECTURAL REQUIREMENTS.

   4.1 ROOF SHEATHING SHALL BE APA RATED SHEATHING, EXPOSURE 1, 48"x96". FOR SUPPORTS 24" OC USE 23/32" 32/16 PLYWOOD. PLYWOOD SHALL BE TONGUE AND GROOVE OR BE INSTALLED WITH PANEL CLIPS IN ACCORDANCE WITH APA RECOMMENDATIONS. WHERE ALLOWABLE SPANS ARE EXCEEDED AT ROOF SLOPE TRANSITIONS, PROVIDE SPECIALLY DESIGNED SUPPLEMENTAL MEMBERS AS REQUIRED. SHEATHING SHALL BE INSTALLED WITH THE LONG EDGE ACROSS A MINIMUM OF THREE SUPPORTING MEMBERS. SUPPORT AND STAGGER EDGES OF PLYWOOD PARALLEL TO SUPPORTING MEMBER. PROVIDE CONTINUOUS BLOCKING AT PERIMETER OF EACH DIAPHRAGM PLANE (INCLUDING ROOF SLOPE TRANSITIONS) AND AROUND OPENINGS. FASTEN SHEATHING WITH 8d NAILS AT 6" OC AT SUPPORTED EDGES UNO AND AT 12" OC AT INTERMEDIATE SUPPORTS. AN 1/8" GAP SHALL BE LEFT BETWEEN ADJACENT PANELS. PROTECT EDGES AGAINST EXPOSURE TO WEATHER OR USE EXTERIOR GRADE PLYWOOD. COVER SHEATHING AS SOON AS POSSIBLE WITH ROOFING FELT OR SHINGLE UNDERLAYMENT FOR PROTECTION AGAINST EXCESSIVE MOISTURE PRIOR TO ROOFING INSTALLATION.

DEFERRED SUBMITTALS

1. DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF PERMIT APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.

2. SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD THROUGH THE ARCHITECT AND GENERAL CONTRACTOR. ONCE THE SUBMITTAL DOCUMENTS HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS, THE ENGINEER OF RECORD WILL FORWARD THEM TO THE ARCHITECT WITH A NOTATION INDICATING THAT THEY ARE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE ARCHITECT WILL FORWARD THE DEFERRED SUBMITTAL DOCUMENTS TO THE GENERAL CONTRACTOR WHO WILL MAINTAIN ONE SET ON SITE FOR REFERENCE BY THE BUILDING INSPECTOR. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

3. DEFERRED SUBMITTALS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, CALCULATIONS, DESIGN LOAD DATA AND SUPPORT REACTIONS OF THE COMPONENTS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE.

4. ITEMS THAT ARE SUBMITTED FOR CONSIDERATION AS DEFERRED SUBMITTALS ARE AS FOLLOWS:

   a. STEEL JOISTS AND JOIST GIRDERS
   b. STEEL CONNECTION DESIGN
   c. PREFABRICATED STEEL STAIRS, HANDRAILS, AND GUARDRAILS
   d. PREFABRICATED CANOPIES
   e. COLD-FORMED METAL FRAMING AND TRUSS SYSTEMS

BUS CANOPY
A = 25 SF
ROOF SURFACES = -39.1/+41.4 PSF

CLASSROOM WING

CLASSROOM WING

CLASSROOM WING
ALTERNATE A: PRE-K WING

CLASSROOM WING

CORE BUILDING

CAFETERIA

GYM

CAFETERIA AWNINGS (TYP)
A = 25 SF
COMBINED UPPER & LOWER SURFACES = -24.7/+24.6 PSF

GYM AWNINGS (TYP)
A = 25 SF
COMBINED UPPER & LOWER SURFACES = -17.1/+26.2 PSF

ENTRY AWNINGS (TYP)
A = 25 SF
COMBINED UPPER & LOWER SURFACES = -16.0/+24.6 PSF

HIGH HIPPED ROOF

ALTERNATE B: PORTE COCHERE EXTENSION

18'-5" TYP
18'-5" TYP
6'-1" TYP
9'-11" TYP
9'-11" TYP
3'-4" TYP
13'-6" TYP
13'-6" TYP
4'-6" TYP

COMPONENTS AND CLADDING ULTIMATE WIND PRESSURE SCHEDULE (PSF)

EFFECTIVE WIND AREA	ROOF				WALL		PARAPET	
	ZONE 1'	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5 (NOTE 2)	ZONE 2	ZONE 3 (NOTE 2)
25 SF	-31.8/+16.0	-50.5/+16.0	-66.8/+16.0	-66.8/+21.9	-32.6/+29.9	-38.7/+29.9	-51.2/+84.9	-57.2/+104.9

SCHEDULE NOTES:
1. (+) AND (-) SIGNS INDICATE PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACE, RESPECTIVELY.
2. AT CLASSROOM WINGS, PRESSURES APPLY **6'-8"** FROM PROMINENT BUILDING CORNER. AT REMAINDER OF BUILDING, PRESSURES APPLY **9'-0"** FROM PORMINENT BUILDING CORNER.
3. JOIST MANUFACTURER MAY DEVELOP WIND PRESSURES BASED ON LARGER TRIBUTARY AREAS, DEAD LOAD FOR UPLIFT CALCULATION TO BE 5 PSF.

COMPONENTS AND CLADDING ULTIMATE WIND PRESSURE SCHEDULE (PSF)

EFFECTIVE WIND AREA	ROOF				WALL		PARAPET	
	ZONE 1'	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5 (NOTE 2)	ZONE 2	ZONE 3 (NOTE 2)
25 SF	-34.0/+16.0	-54.0/+16.0	-71.4/+32.0	-71.4/+32.0	-34.8/+32.0	-41.3/+32.0	-54.8/+90.8	-61.2/+112.2

SCHEDULE NOTES:
1. (+) AND (-) SIGNS INDICATE PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACE, RESPECTIVELY.
2. PRESSURES APPLY **10'-2"** FROM PROMINENT BUILDING CORNER IN EACH DIRECTION.
3. JOIST MANUFACTURER MAY DEVELOP WIND PRESSURES BASED ON LARGER TRIBUTARY AREAS, DEAD LOAD FOR UPLIFT CALCULATION TO BE 5 PSF.

ABBREVIATIONS

ABT	ABOUT
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL/ARCHITECT
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS
AWS	AMERICAN WELDING SOCIETY
B	BOTTOM OF
BLDG	BUILDING
BM	BEAM
BOTT	BOTTOM
BRG	BEARING
BTWN	BETWEEN
C/C	CENTER TO CENTER
CALC	CALCULATION(S)
CHKD	CHECKED
CIP	CAST-IN-PLACE CONCRETE
CJ	CONSTRUCTION CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLR	CLEAR, CLEARANCE
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
COORD	COORDINATE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
CTR	CENTER
CTRD	CENTERED
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DEG	DEGREES
DET	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIR	DIRECTION
DL	DEAD LOAD
DWG	DRAWING
(E)	EXISTING
EA	EACH
EF	EACH FACE
EL	ELEVATION
ELEC	ELECTRICAL
ENGR	ENGINEER
EOD	EDGE OF DECK
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FINISHED FLOOR
FG	FINISHED GRADE
FIN	FINISH
FLG	FLANGE
FLR	FLOOR
FRMG	FRAMING
FRP	FIBER REINFORCED PLASTIC
FS	FAR SIDE
FT	FOOT
FTG	FOOTING
FV	FIELD VERIFY
GA	GAGE, GAUGE
GALV	GALVANIZED (HOT DIP)
GRTG	GRATING
(H)	HORIZONTAL BEAM ORIENTATION
HCA	HEADED CONCRETE ANCHOR
HDR	HEADER
HGR	HANGER
HORIZ	HORIZONTAL
HP	HIGH POINT
HR	HANDRAIL
HSB	HIGH STRENGTH BOLT
IF	INSIDE FACE
IBC	INTERNATIONAL BUILDING CODE
ICC	INTERNATIONAL CODE COUNCIL
ID	INSIDE DIAMETER
IE	INVERT ELEVATION
IN	INCH
INT	INTERIOR
JT	JOINT
K	KIP(S)
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
(LLH)	LONG LEG HORIZONTAL (ANGLE)
(LLV)	LONG LEG VERTICAL (ANGLE)
(LSH)	LONG SIDE HORIZONTAL (HSS)
(LSV)	LONG SIDE VERTICAL (HSS)
LB	POUND
LF	LINEAR FEET
LL	LIVE LOAD
LOC	LOCATION
LONG	LONGITUDINAL
LP	LOW POINT
LSH	LONG SLOTTED HOLE
M	MOMENT
MAX	MAXIMUM
MC	MOMENT CONNECTION
MECH	MECHANICAL
MFC	MANUFACTURED
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MTD	MOUNTED
MTL	METAL
N&F	NEAR AND FAR
N/A	NOT APPLICABLE
NIC	NOT IN CONTRACT
NO/NO.	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
OSH	OVERSIZED HOLE
PCF	POUNDS PER CUBIC FOOT
PE	PROFESSIONAL ENGINEER
PERIM	PERIMETER
PJF	PREMOLDED JOINT FILLER
PL	PLATE
PLCS	PLACES
PLF	POUNDS PER LINEAR FOOT
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT
R	RADIUS
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
REQD	REQUIRED
RET	RETURN
REV	REVISION
RO	ROUGH OPENING
RTU	ROOFTOP UNIT
SC	SLIP CRITICAL
SCHED	SCHEDULE
SECT	SECTION
SHT	SHEET
SIM	SIMILAR
SL	SLOPE
SPCS	SPACES
SPEC(S)	SPECIFICATION(S)
SQ	SQUARE
SS	STAINLESS STEEL
SSH	SHORT SLOTTED HOLE
STD	STANDARD
STIF	STIFFENER
STL	STEEL
STRUCT	STRUCTURAL
SYM	SYMMETRICAL
T&B	TOP AND BOTTOM
T/O	TOP OF
THK	THINK
THRU	THROUGH
TYP	TYPICAL
UL	UNDERWRITER'S LABORATORIES
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WITH
W/O	WITHOUT
WP	WORKING POINT
WS	WATERSTOP
WWF	WELDED WIRE FABRIC

2 S0.02 ABBREVIATIONS
3/4" = 1'-0"

STEEL COLUMN/FOOTING TYPE INDICATOR

COL	SIZE STEEL COLUMN SIZE
BP-X	BASE PLATE MARK
F.X.X (-0'-0")	FOOTING MARK (T/FTG ELEVATION)
P-X (-0'-0")	PEDESTAL MARK (T/PEDESTAL ELEVATION)

FOUNDATION STEP INDICATOR
(-x'-x") T/FOOTING ELEVATION
(-x'-x") T/FOOTING ELEVATION

OPENING IN FLOOR OR ROOF

RECESS/DEPRESSION INDICATOR

STRUCTURAL STEEL CONNECTION AXIAL FORCE
(xxk)

SLOPE INDICATOR
Y
X

CONCRETE SLAB/METAL DECK SPAN INDICATOR

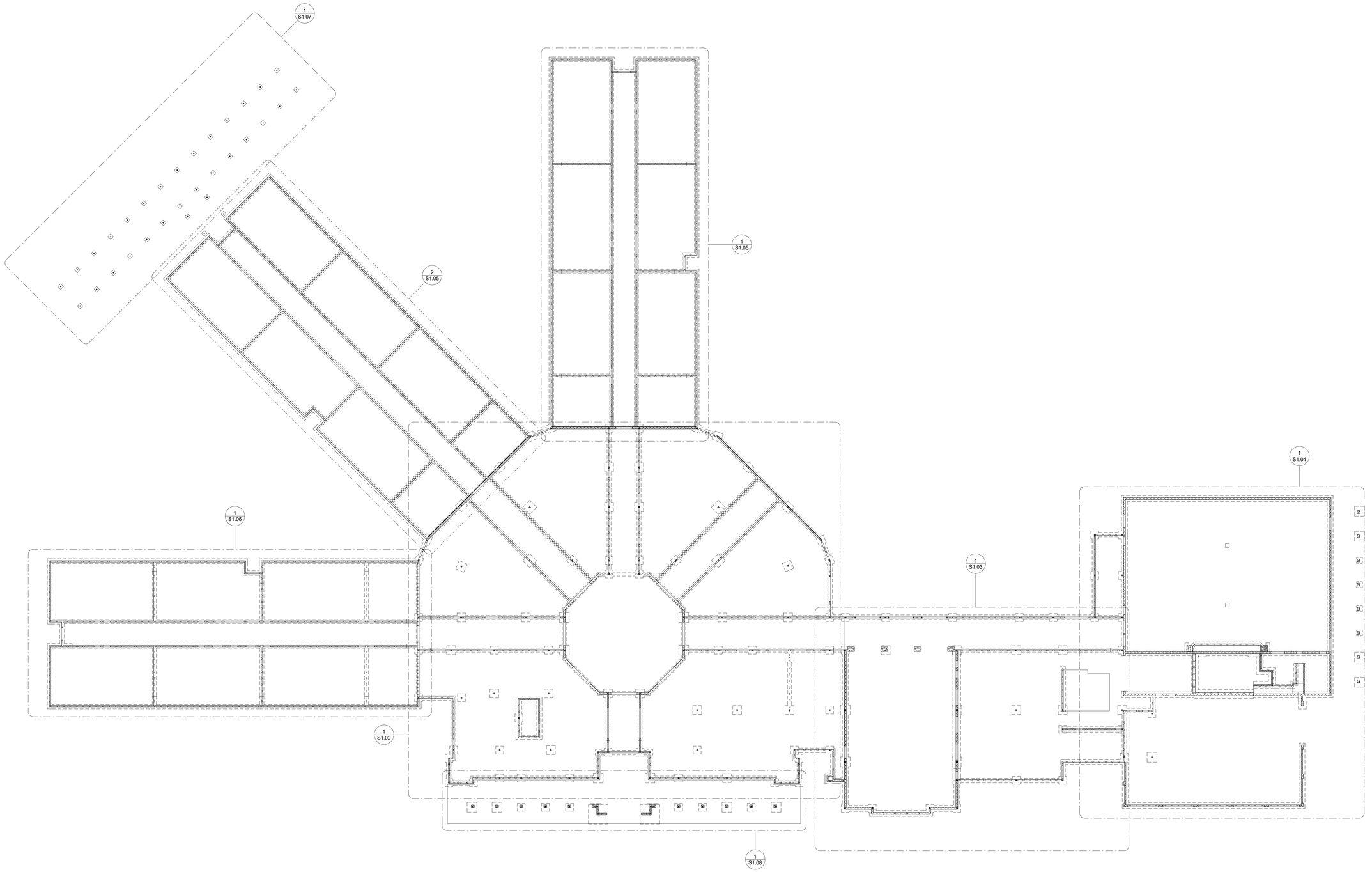
STRUCTURAL STEEL MOMENT CONNECTION

STRUCTURAL STEEL BEAM DESIGNATION
BEAM SIZE
NUMBER OF STUDS UNIFORMLY SPACED ALONG BEAM
W8x10 (XX) c=x"
(XXk) (XXk)
BEAM CAMBER
BEAM SERVICE REACTION

STRUCTURAL STEEL BEAM SPLICE DESIGNATION
CL SPLICE

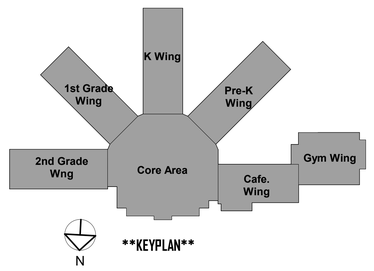
STRUCTURAL CONCRETE BEAM DESIGNATION
CBX INDICATES CONVENTIONALLY REINFORCED CONCRETE BEAM MARK
BEAM WIDTH MAY VARY BASED UPON EDGE OF SLAB DIMENSION
CB XXX PB XXX
PBX INDICATES POST-TENSIONED CONCRETE BEAM MARK, BEAM WIDTH MAY VERY BASED UPON EDGE OF SLAB DIMENSION

1 S0.02 STRUCTURAL LEGEND
3/4" = 1'-0"



1 S1.01 Foundation Plan - Main Level - Overall
1" = 20'-0"

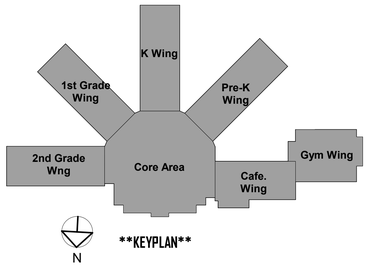
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.



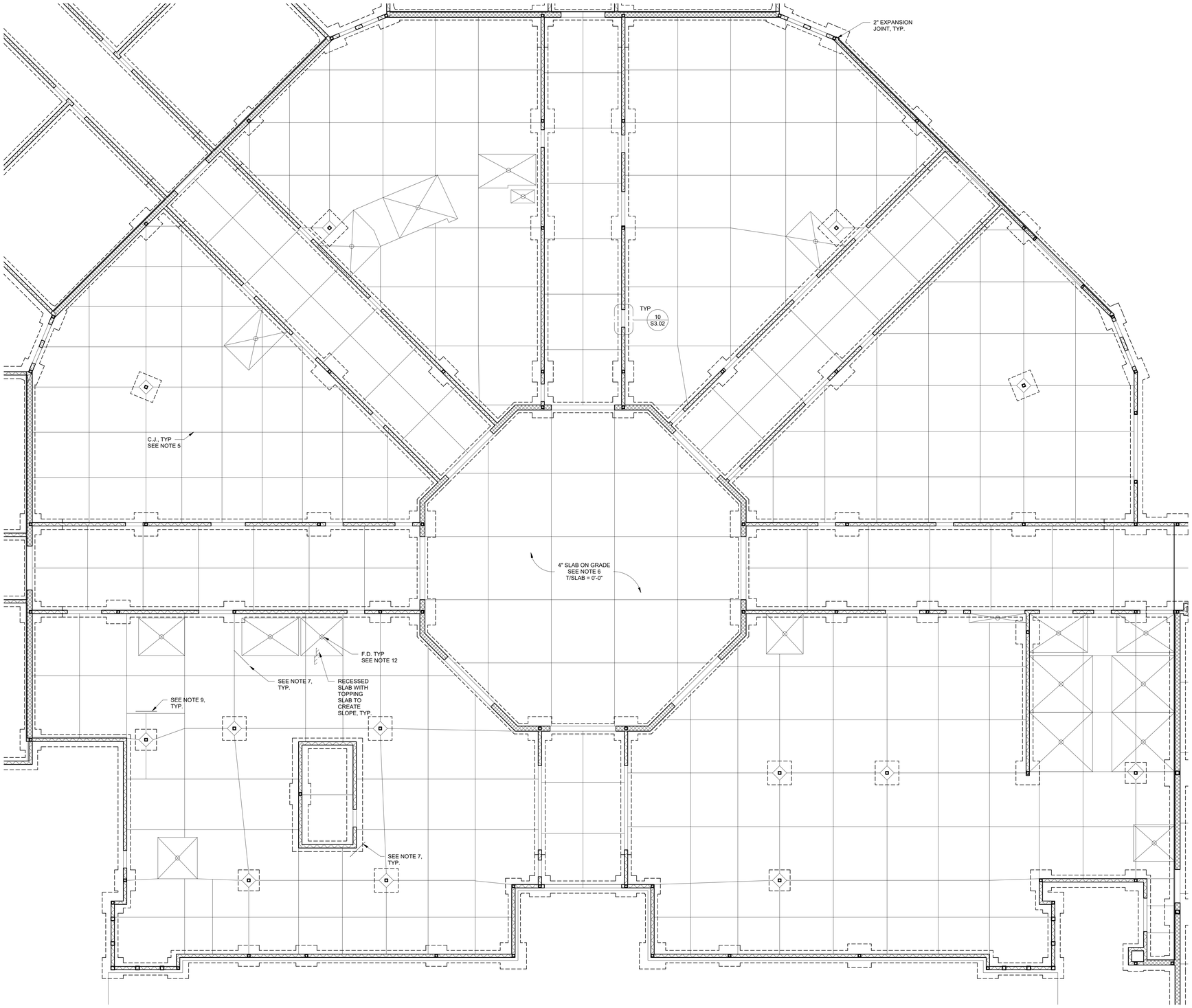
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. ⟋ INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. INDICATES SLAB DEPRESSION. SEE 12/S3.02.
8. INDICATES REINFORCED 8" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
9. INDICATES REINFORCED 12" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
10. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
12. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
13. MSW INDICATES INTERIOR MASONRY SHEAR WALL, SEE DETAIL 17/S4.01. ALL EXTERIOR MASONRY WALLS SHOWN ARE SHEAR WALLS. (2) CELLS GROUTED AND REINF. WITH #5 VERT TO BE PROVIDED AT EA END, UNO.

MASONRY WALL REINFORCING SCHEDULE

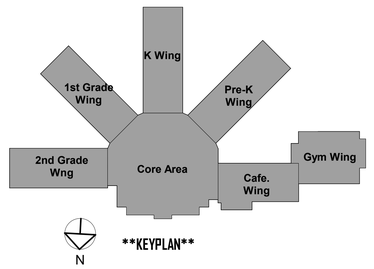
INTERIOR WALLS			
8" CMU WALLS		#4@ 48" O.C.	CENTERED IN WALL
12" CMU WALLS		#5@ 32" O.C.	CENTERED IN WALL
EXTERIOR WALLS			
8" CMU WALLS	HT < 20'-0"	#5@ 24" O.C.	CENTERED IN WALL
	HT ≥ 20'-0"	#5@ 16" O.C.	CENTERED IN WALL
12" CMU WALLS	HT < 24'-0"	#5@ 32" O.C.	CENTERED IN WALL
	HT ≥ 24'-0"	#5@ 32" O.C.	DOUBLE LAYER



1 S1.02 Foundation Plan - Main Level - Core Area
1/8" = 1'-0"



NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
5. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
6. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
7. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
8. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS. SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
9. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
10. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
11. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.
12. F.D. INDICATES FLOOR DRAIN. SLOPE SLAB TO DRAIN.
13. ▨ INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02.

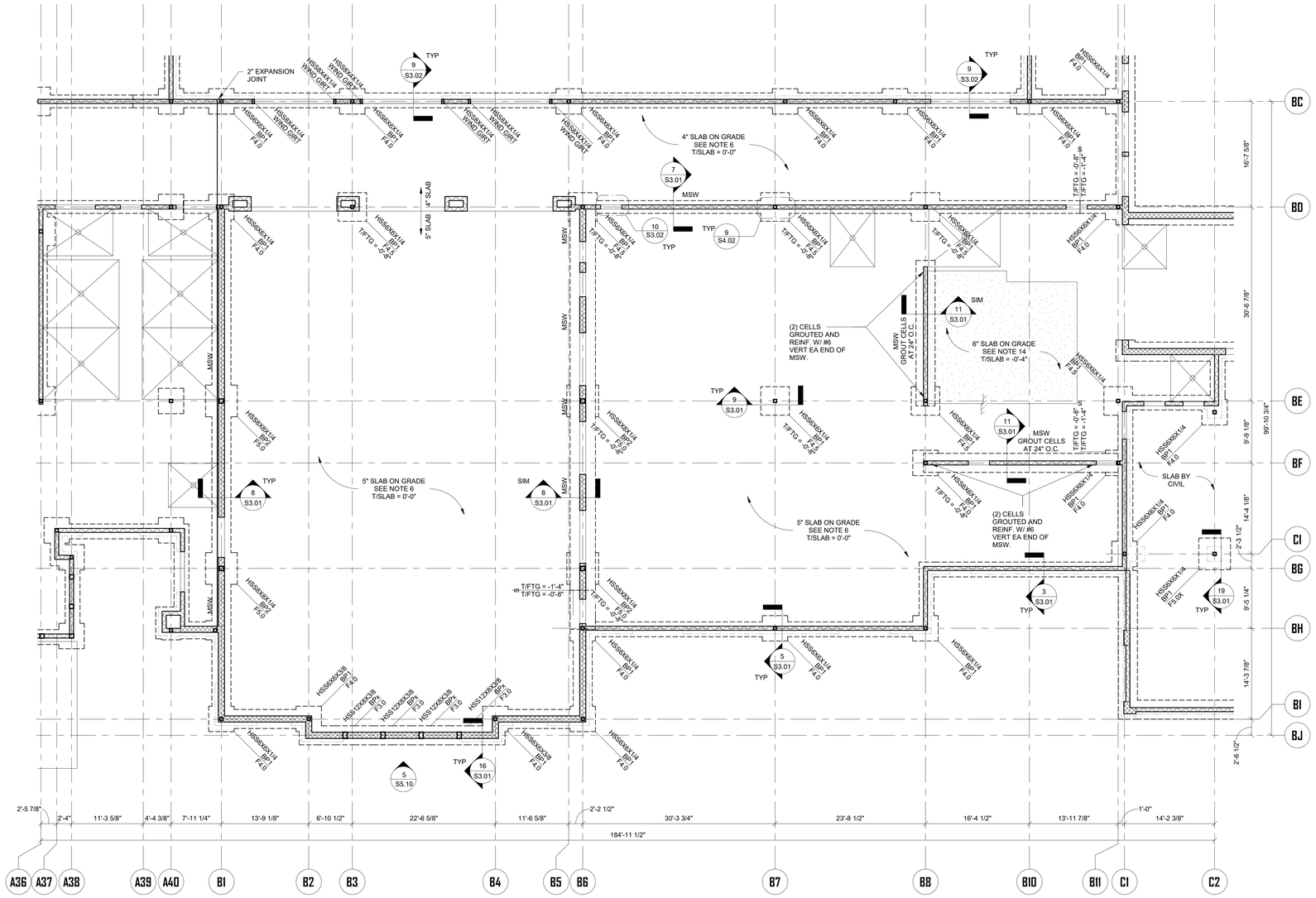


1 / S1.02a Slab Plan - Main Level - Core Area
1/8" = 1'-0"

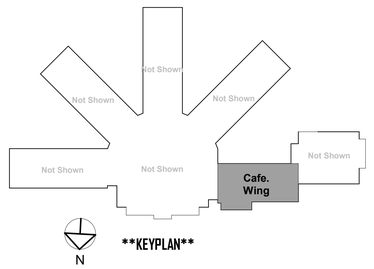
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" OR 5" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. INDICATES REINFORCED 8" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
8. INDICATES REINFORCED 12" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
9. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
10. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
11. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
12. MSW INDICATES INTERIOR MASONRY SHEAR WALL, SEE DETAIL 17/S4.01. ALL EXTERIOR MASONRY WALLS SHOWN ARE SHEAR WALLS.(2) CELLS GROUTED AND REINF. WITH #5 VERT TO BE PROVIDED AT EA END, UNO.
13. INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02 .
14. PROVIDE RECESSED 6" SLAB ON GRADE REINFORCED WITH WWF6x6 W2.9xW2.9 ON VAPOR BARRIER AND 6" GRANULAR BASE.

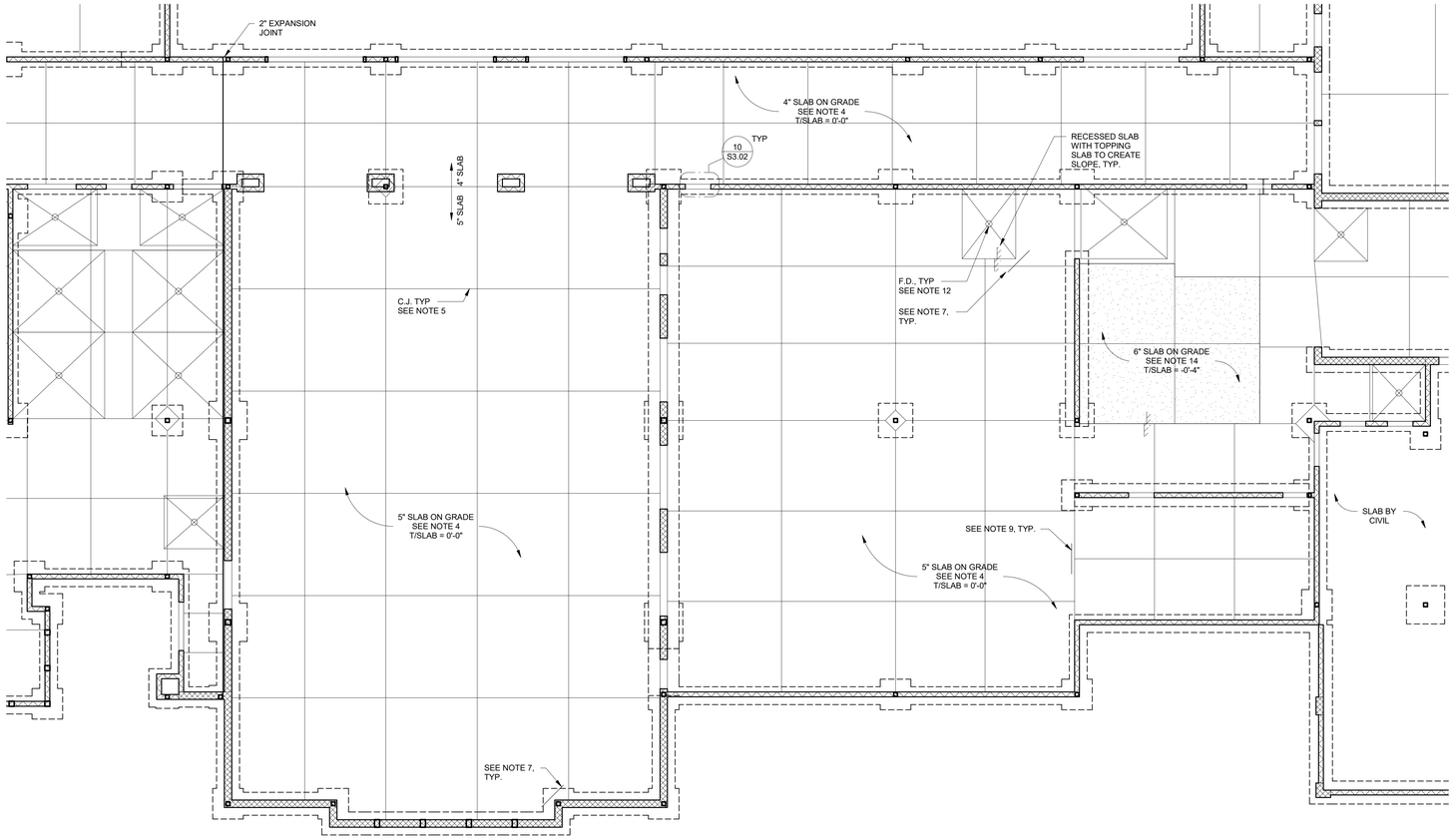
MASONRY WALL REINFORCING SCHEDULE

INTERIOR WALLS			
8" CMU WALLS		#4@ 48" O.C.	CENTERED IN WALL
12" CMU WALLS		#5@ 32" O.C.	CENTERED IN WALL
EXTERIOR WALLS			
8" CMU WALLS	HT < 20'-0"	#5@ 24" O.C.	CENTERED IN WALL
	HT ≥ 20'-0"	#5@ 16" O.C.	CENTERED IN WALL
12" CMU WALLS	HT < 24'-0"	#5@ 32" O.C.	CENTERED IN WALL
	HT ≥ 24'-0"	#5@ 32" O.C.	DOUBLE LAYER



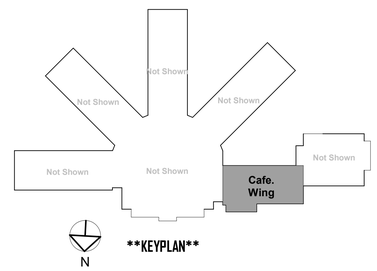
1 S1.03 Foundation Plan - Main Level - Cafe. Wing
1/8" = 1'-0"

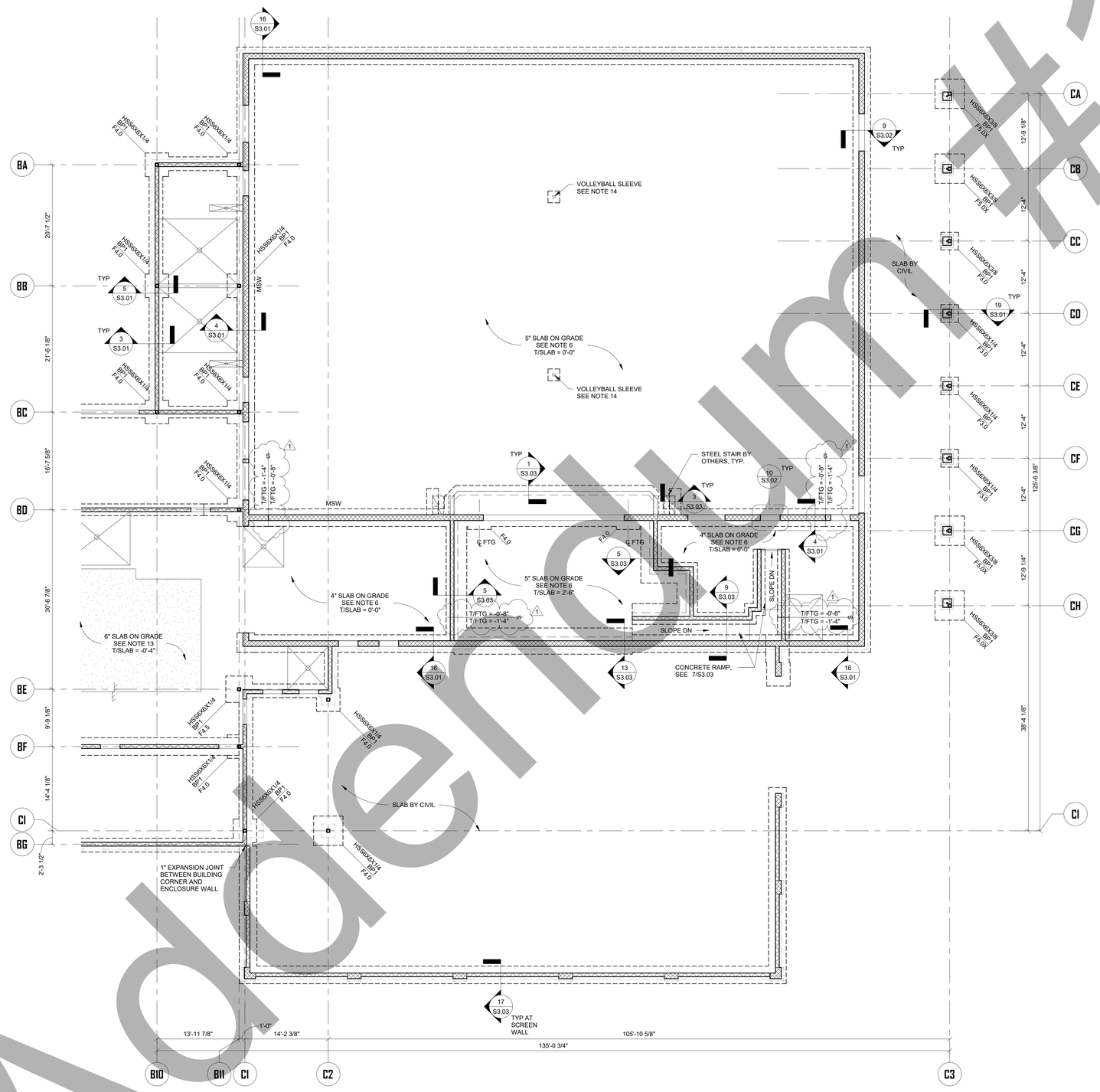




1 S1.03a Slab Plan - Main Level - Cafe. Wing
1/8" = 1'-0"

NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. PROVIDE 4" OR 5" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
5. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
6. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02 .
7. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
8. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS. SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
9. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
10. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
11. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.
12. F.D. INDICATES FLOOR DRAIN. SLOPE SLAB TO DRAIN.
13. INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02 .
14. PROVIDE RECESSED 6" SLAB ON GRADE REINFORCED WITH WWF6x6 W2.9xW2.9 ON VAPOR BARRIER AND 6" GRANULAR BASE.



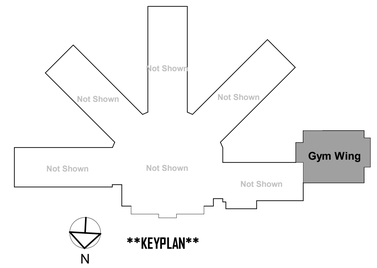


1 S1.04 Foundation Plan - Main Level - Gym Wing
1/8" = 1'-0"

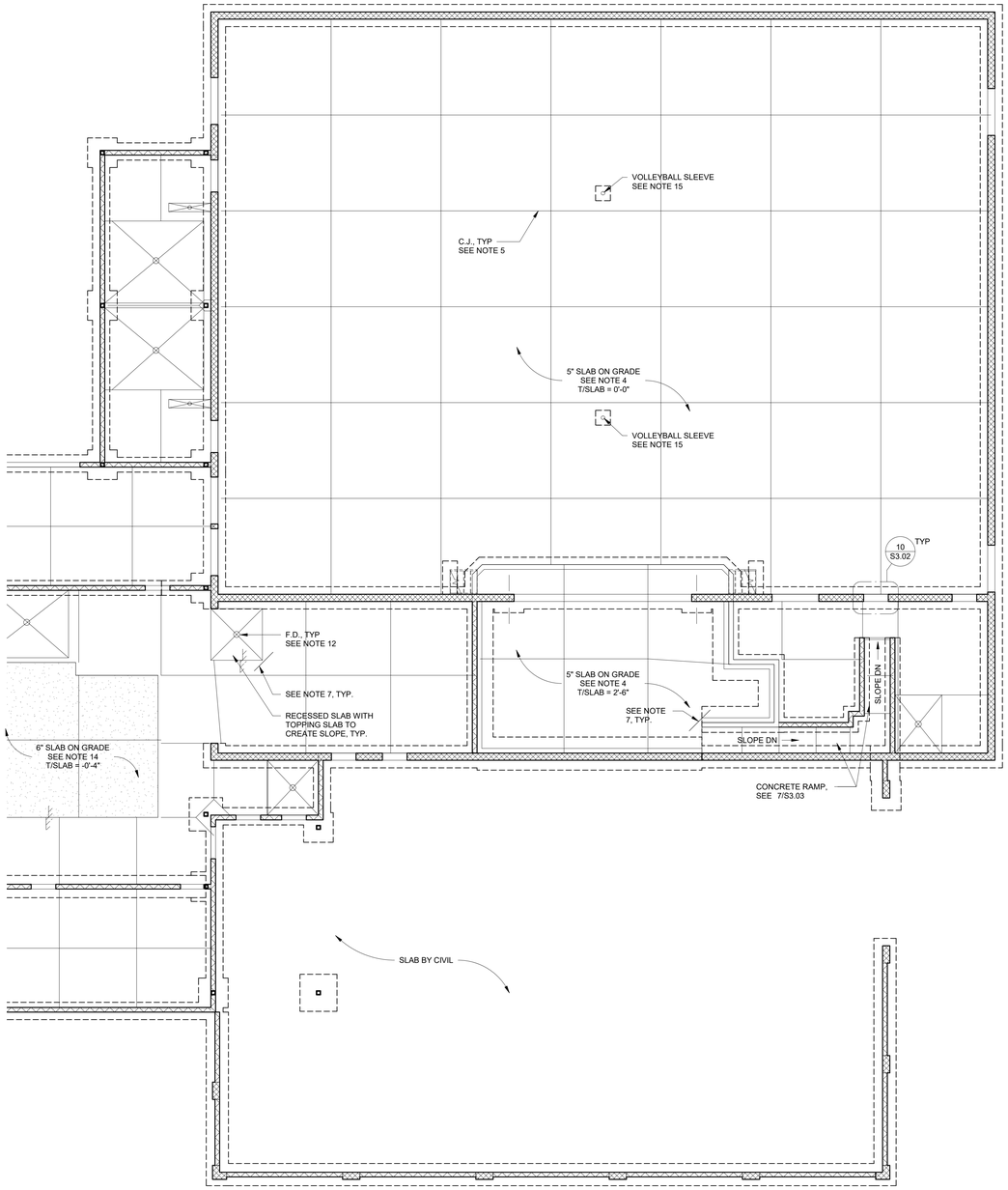
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. ⟡ INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" OR 6" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. ▯▯▯ INDICATES REINFORCED 8" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
8. ▯▯▯▯ INDICATES REINFORCED 12" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
9. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
10. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
11. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS. SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
12. MSW INDICATES INTERIOR MASONRY SHEAR WALL. SEE DETAIL 17/S4.01. ALL EXTERIOR MASONRY WALLS SHOWN ARE SHEAR WALLS.
13. PROVIDE RECESSED 6" SLAB ON GRADE REINFORCED WITH WWF6x6 W2.9xW2.9 ON VAPOR BARRIER AND 6" GRANULAR BASE.
14. VOLLEYBALL SLEEVE, COORDINATE LOCATION WITH ARCH. SEE DETAIL 14/S3.02 .
15. ▱▱▱ INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02.

MASONRY WALL REINFORCING SCHEDULE

INTERIOR WALLS			
8" CMU WALLS		#4@ 48" O.C.	CENTERED IN WALL
12" CMU WALLS		#5@ 32" O.C.	CENTERED IN WALL
EXTERIOR WALLS			
8" CMU WALLS	HT < 10'-0"	#5@ 32" O.C.	CENTERED IN WALL
	10'-0" ≤ HT < 20'-0"	#5@ 24" O.C.	CENTERED IN WALL
	HT ≥ 20'-0"	#5@ 16" O.C.	CENTERED IN WALL
12" CMU WALLS	HT < 24'-0"	#5@ 32" O.C.	CENTERED IN WALL
	HT ≥ 24'-0"	#5@ 32" O.C.	DOUBLE LAYER

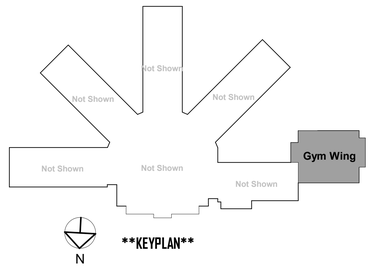


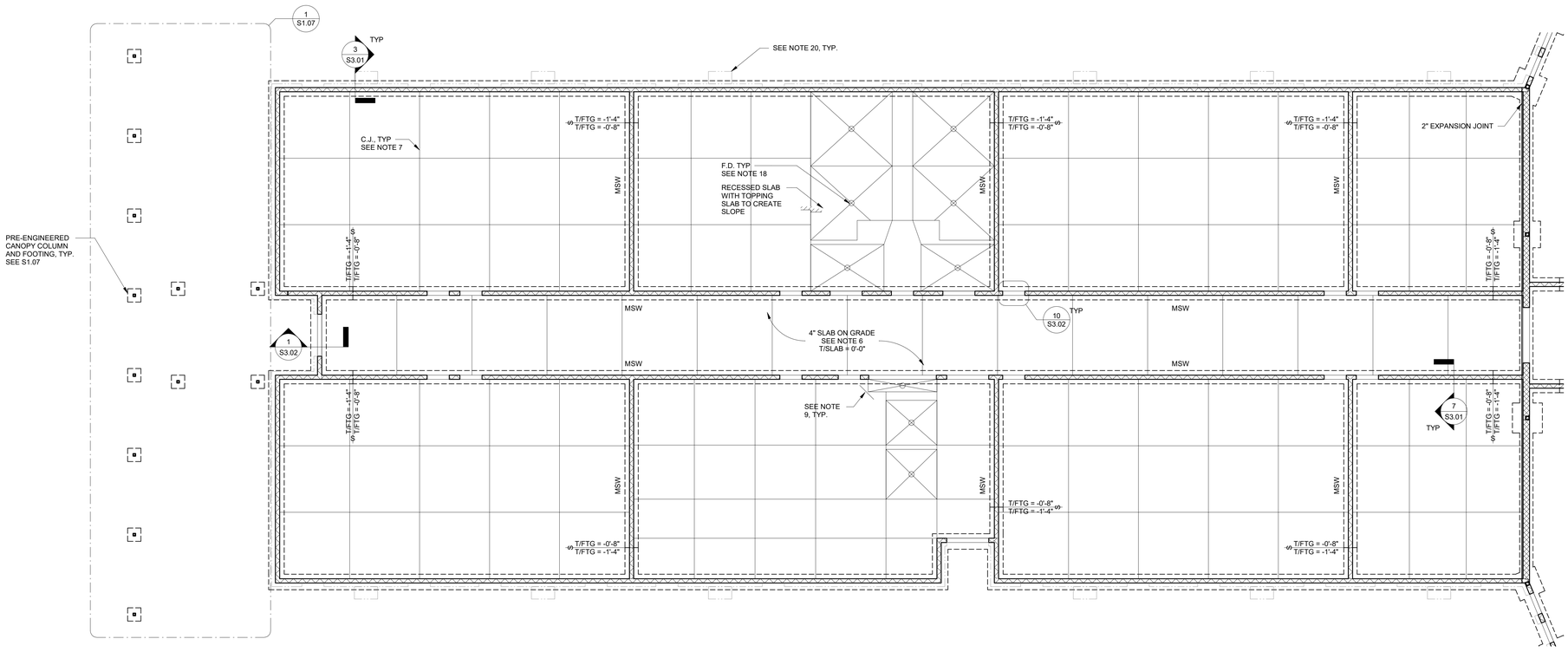
KEYPLAN



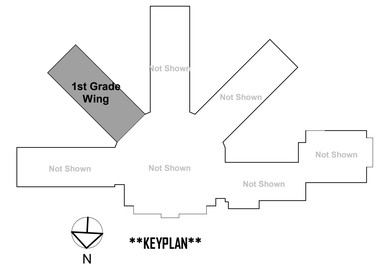
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. PROVIDE 4" OR 5" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
5. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
6. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
7. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
8. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
9. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
10. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
11. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.
12. F.D. INDICATES FLOOR DRAIN. SLOPE SLAB TO DRAIN.
13. INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02.
14. PROVIDE RECESSED 6" SLAB ON GRADE REINFORCED WITH WWF6x6 W2.9xW2.9 ON VAPOR BARRIER AND 6" GRANULAR BASE.
15. VOLLEYBALL SLEEVE, COORDINATE LOCATION WITH ARCH. SEE DETAIL 14/S3.02.

1 / S1.04a — Slab Plan - Main Level - Gym Wing
1/8" = 1'-0"

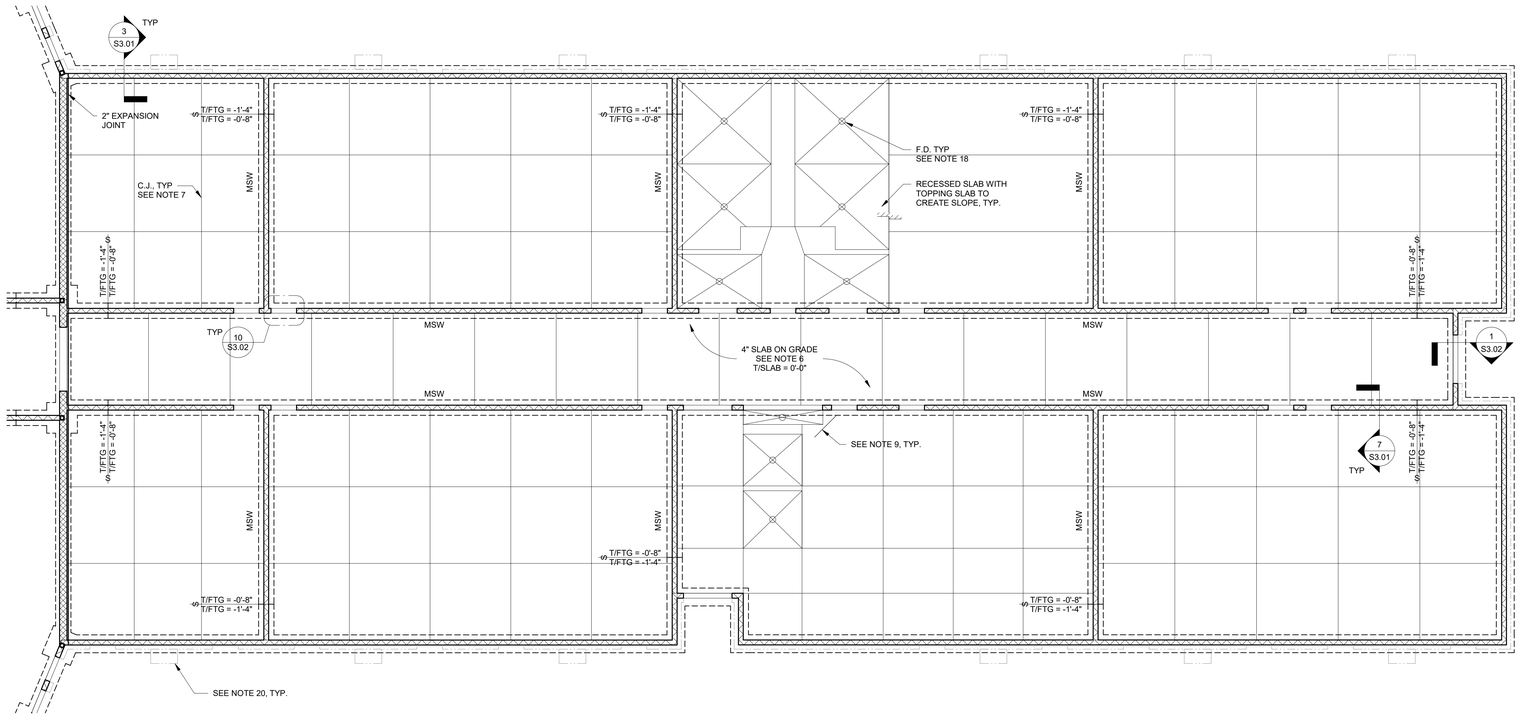




2 S1.05 Foundation Plan - Main Level - 1st Grade Wing
1/8" = 1'-0"



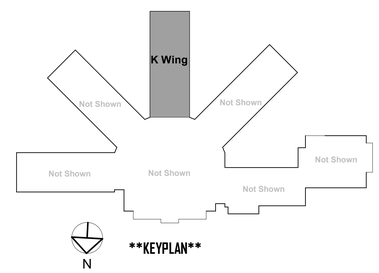
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. ⟲ INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
8. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
9. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
10. ▭ INDICATES REINFORCED 8" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
11. ▭ INDICATES REINFORCED 12" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
12. SEE DETAILS 5/S4.01 AND 9/S4.01FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
14. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
15. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
16. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
17. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.
18. F.D. INDICATES FLOOR DRAIN. SLOPE SLAB TO DRAIN.
19. MSW INDICATES INTERIOR MASONRY SHEAR WALL, SEE DETAIL 17/S4.01. ALL EXTERIOR MASONRY WALLS SHOWN ARE SHEAR WALLS.
20. PROVIDE MASONRY AND BRICK LINTELS AT OPENINGS FOR WALL MOUNTED HEAT PUMPS. CONTRACTOR TO COORDINATE SIZE AND LOCATION WITH MECHANICAL. SEE 2/S4.01 FOR LINTEL SCHEDULE.
21. ▱ INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02 .



1 S1.05 Foundation Plan - Main Level - K Wing
1/8" = 1'-0"

MASONRY WALL REINFORCING SCHEDULE

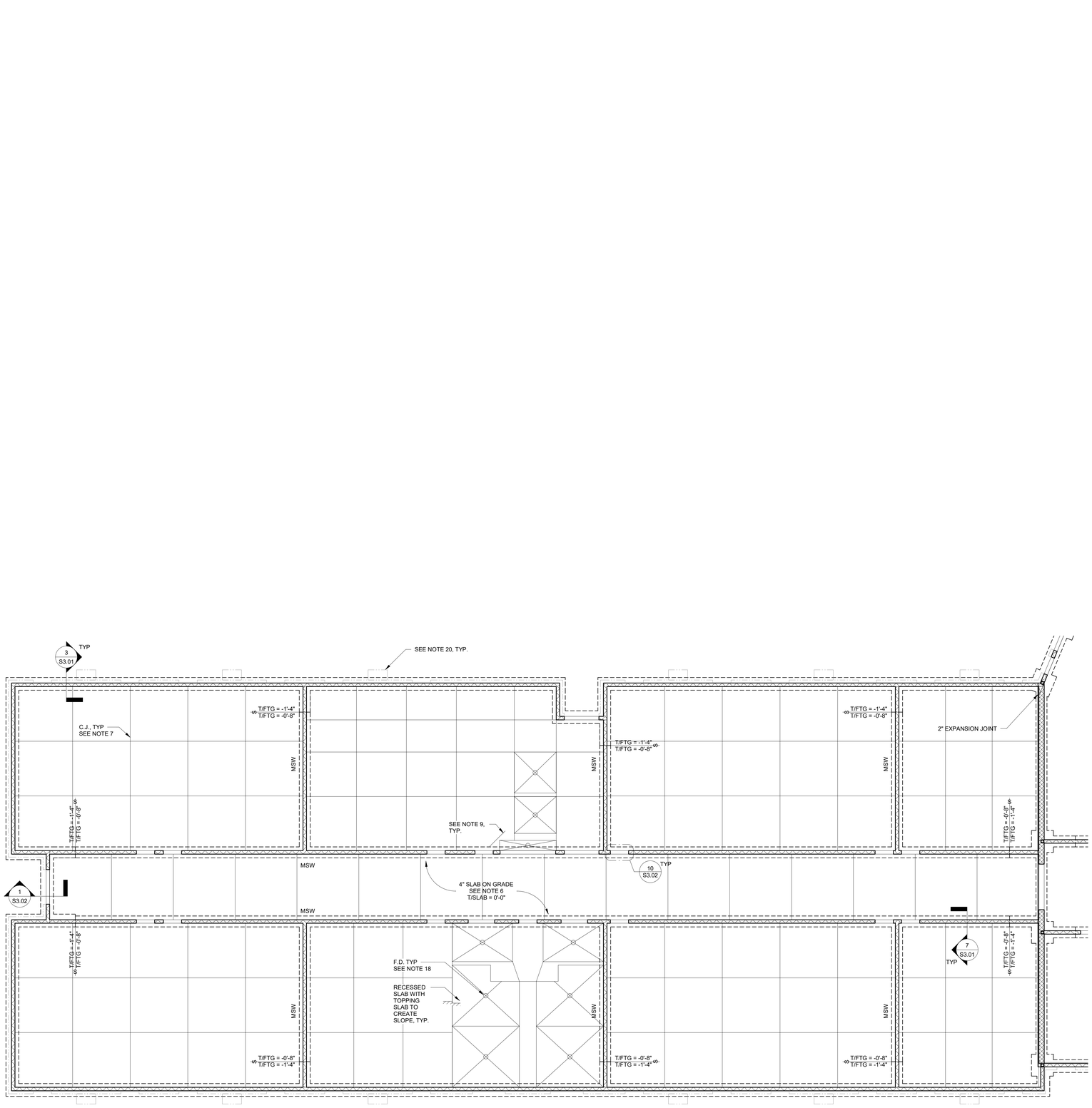
INTERIOR WALLS			
8" CMU WALLS		#4@ 48" O.C.	CENTERED IN WALL
12" CMU WALLS		#5@ 32" O.C.	CENTERED IN WALL
EXTERIOR WALLS			
8" CMU WALLS	HT < 20'-0"	#5@ 32" O.C.	CENTERED IN WALL
12" CMU WALLS	HT < 24'-0"	#5@ 32" O.C.	CENTERED IN WALL
	HT ≥ 24'-0"	#5@ 32" O.C.	DOUBLE LAYER

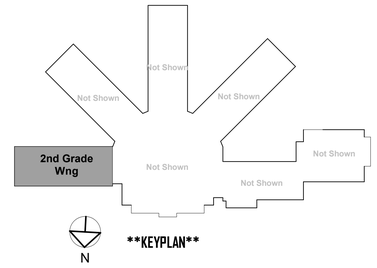


NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 4" GRANULAR BASE.
7. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
8. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
9. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
10. INDICATES REINFORCED 8" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
11. INDICATES REINFORCED 12" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
12. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
14. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
15. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
16. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
17. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.
18. F.D. INDICATES FLOOR DRAIN. SLOPE SLAB TO DRAIN.
19. MSW INDICATES INTERIOR MASONRY SHEAR WALL, SEE DETAIL 17/S4.01. ALL EXTERIOR MASONRY WALLS SHOWN ARE SHEAR WALLS.
20. PROVIDE MASONRY AND BRICK LINTELS AT OPENINGS FOR WALL MOUNTED HEAT PUMPS. CONTRACTOR TO COORDINATE SIZE AND LOCATION WITH MECHANICAL. SEE 2/S4.01 FOR LINTEL SCHEDULE.
21. INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02 .

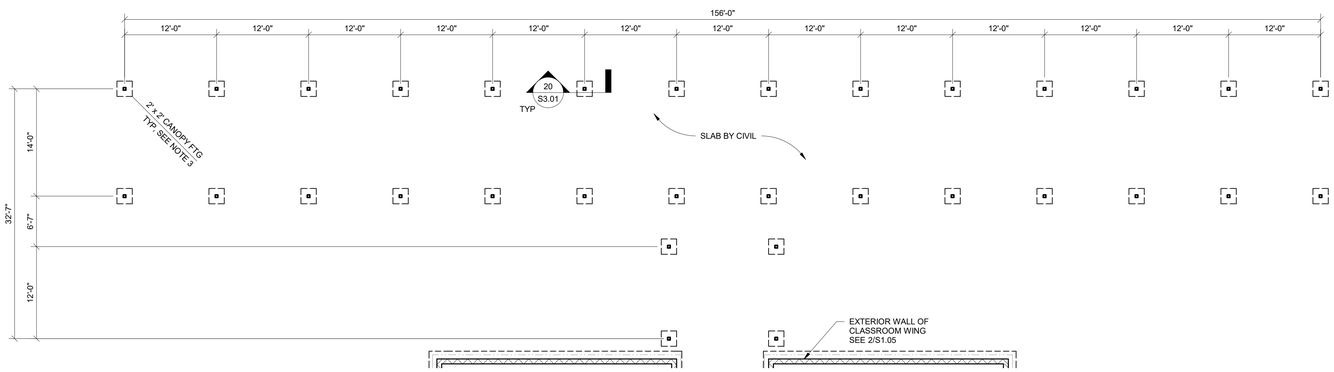
MASONRY WALL REINFORCING SCHEDULE

INTERIOR WALLS			
8" CMU WALLS		#4@ 48" O.C.	CENTERED IN WALL
12" CMU WALLS		#5@ 32" O.C.	CENTERED IN WALL
EXTERIOR WALLS			
8" CMU WALLS	HT < 20'-0"	#5@ 32" O.C.	CENTERED IN WALL
12" CMU WALLS	HT < 24'-0"	#5@ 32" O.C.	CENTERED IN WALL
	HT ≥ 24'-0"	#5@ 32" O.C.	DOUBLE LAYER



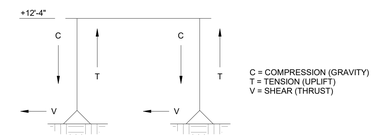


1 S1.06 Foundation Plan - Main Level - 2nd Grade
1/8" = 1'-0"



1 S1.07 Foundation Plan - Main Level - Bus Canopy
1/8" = 1'-0"

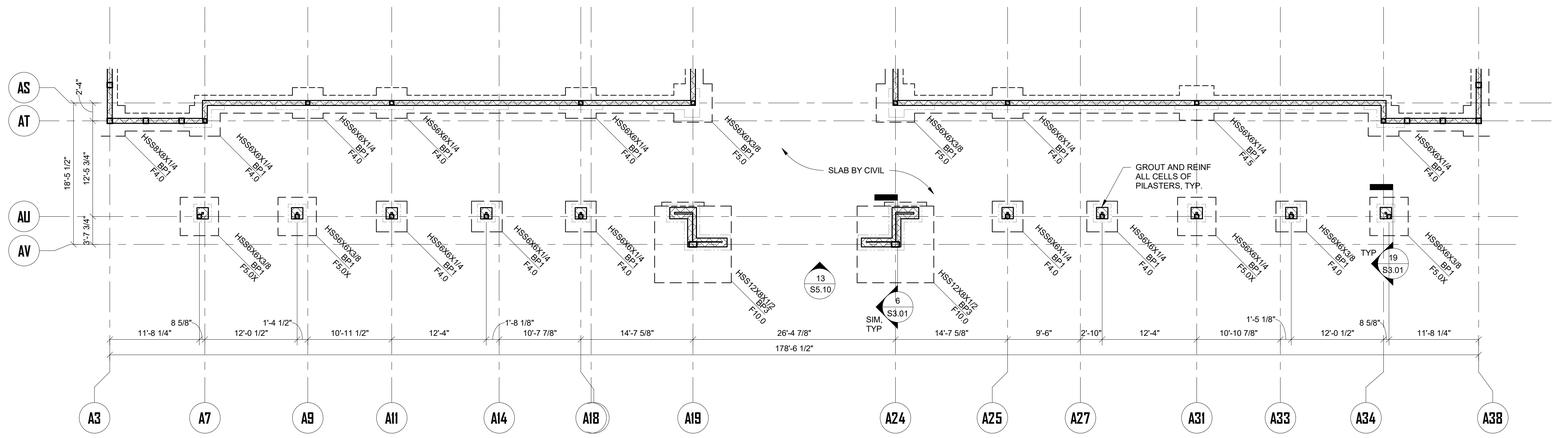
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. CANOPY FOUNDATIONS ARE PRELIMINARY PENDING REACTIONS FROM PRE-ENG CANOPY SUPPLIER. SEE 20/S3.01.
4. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
5. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
6. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.



PRE-ENGINEERED CANOPY
ALLOWABLE DESIGN LOADS PER COLUMN (MAXIMUM)

	GRAVITY (C)	UPLIFT (T)	SHEAR (V)
ALLOWABLE LOAD	1.47 KIPS	-1.74 KIPS	.29 KIPS

*LOADS TO BE VERIFIED W/ PRE-ENGINEERED CANOPY MANUFACTURER PRIOR TO CONSTRUCTION.

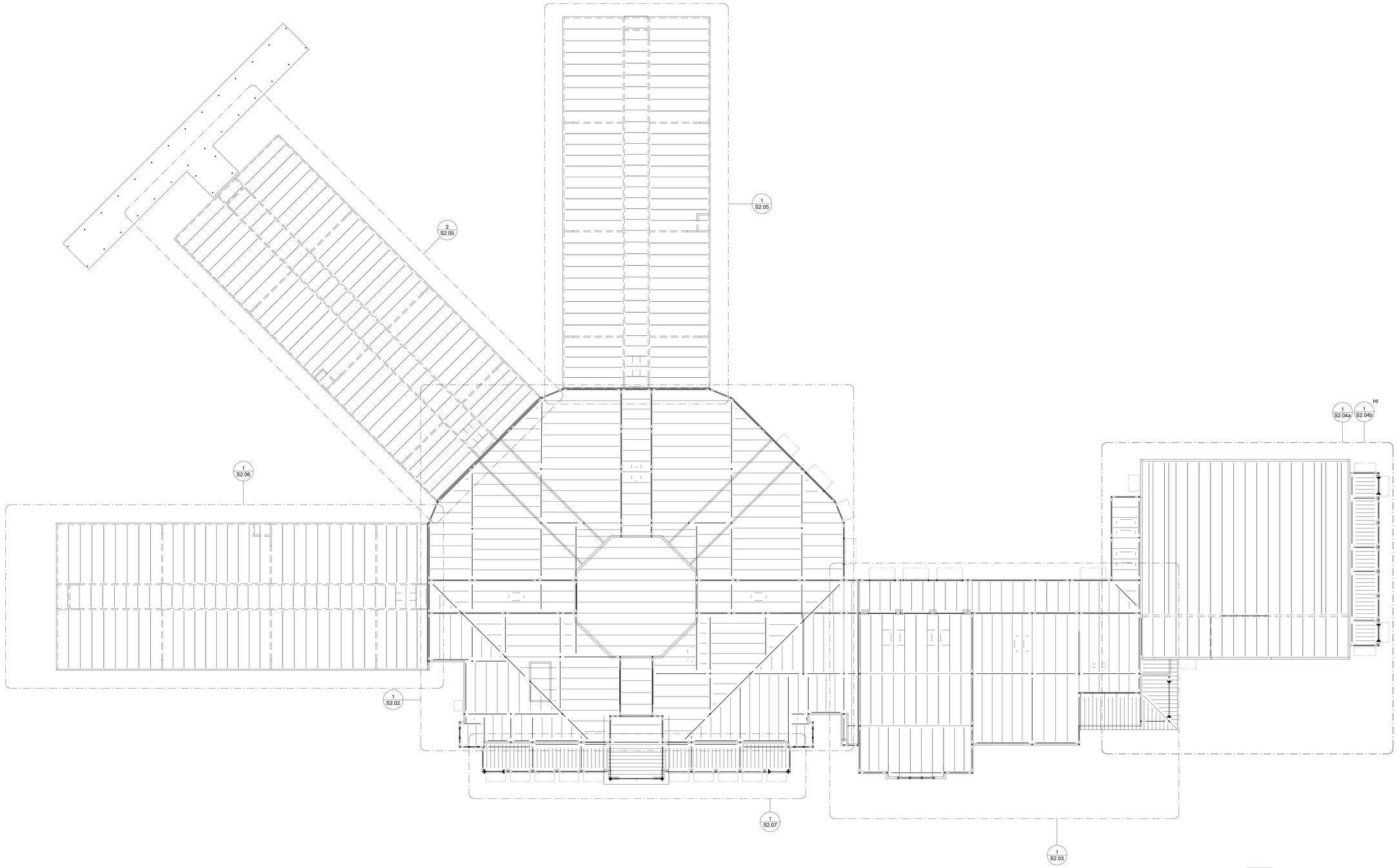


1 / S1.08 Foundation Plan - Main Level - Enlarged Front Drop Off Area

1/8" = 1'-0"

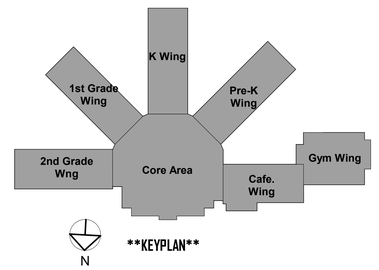
NOTES:

1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/SS.01.
5. INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
8. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
9. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
10. INDICATES 8" MASONRY WALL REINFORCED W/ #5@32" O.C. SEE DETAIL 1/S4.01.
11. INDICATES 12" MASONRY WALL REINFORCED W/ #5@32" O.C. SEE DETAIL 1/S4.01.
12. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
14. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
15. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
16. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
17. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.

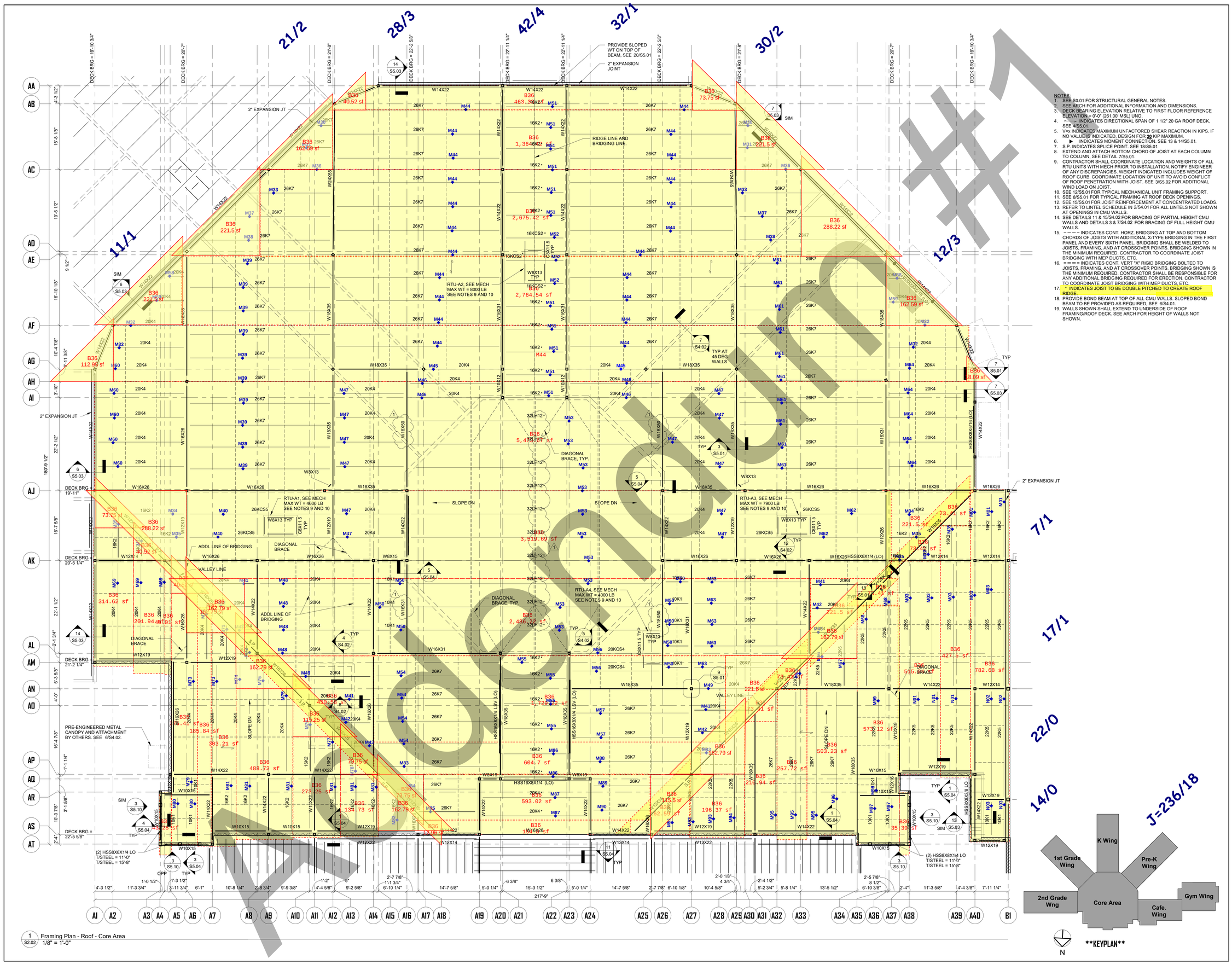


1 S2.01 Framing Plan - Roof - Overall
1" = 20'-0"

NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.

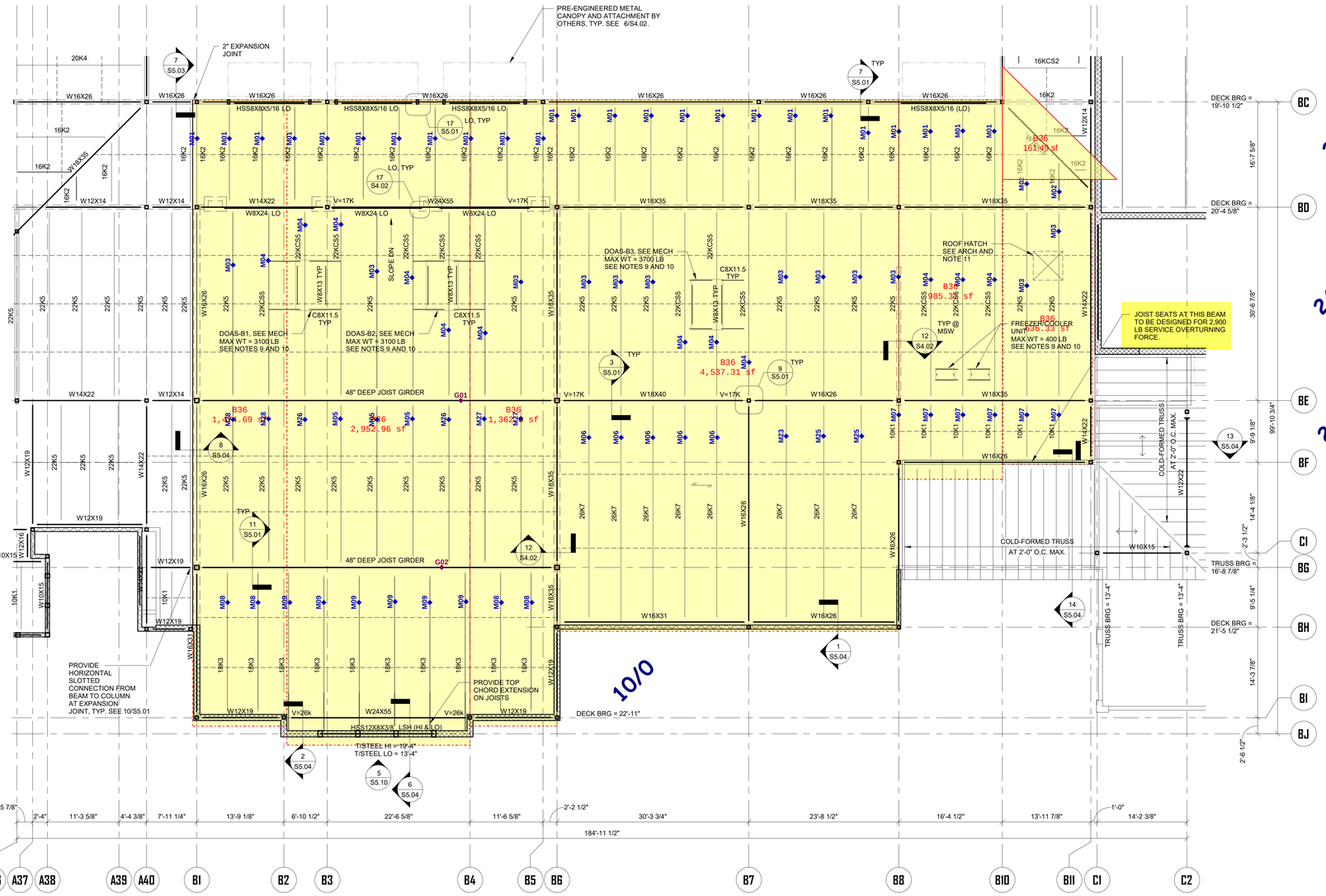


Joist Description	Quantity
M01	3
M03	5
M12	2
M30	2
M31	2
M32	4
M33	2
M34	2
M35	5
M36	2
M37	2
M38	2
M39	10
M40	2
M41	4
M42	4
M43	4
M44	22
M45	2
M46	4
M47	12
M48	4
M49	2
M50	8
M51	11
M52	3
M53	10
M54	4
M55	4
M56	2
M57	2
M58	2
M59	2
M60	5
M61	10
M62	2
M63	5
M64	5
M65	1
M66	1
M67	1
M68	2
M69	3
M70	1
M71	1
M72	1
M73	2
M74	1
M75	2
M76	1
M77	1
M78	1
M79	1
M80	2
M81	6
M82	1
M83	1
M84	1
M85	1
M86	2
M87	2
M88	1
M89	1
M90	1
M91	1
M92	1
M93	1
M94	1
M95	3
M96	1
M97	2
M98	1
M99	1
N01	3
N02	2
N03	2



1 S2.02 Framing Plan - Roof - Core Area
1/8" = 1'-0"

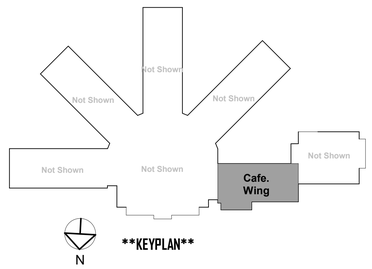
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. ~~~~ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 4/S5.01.
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR 20 KIP MAXIMUM.
6. ▶ INDICATES MOMENT CONNECTION. SEE 13 & 14/S5.01.
7. S.P. INDICATES SPLICE POINT. SEE 18/S5.01.
8. EXTEND AND ATTACH BOTTOM CHORD OF JOIST AT EACH COLUMN TO COLUMN, SEE DETAIL 7/S5.01.
9. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
10. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
11. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
12. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
13. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
14. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
15. ~ ~ ~ ~ INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IN THE MINIMUM REQUIRED. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
16. = = = = INDICATES CONT. VERT "X" RIGID BRIDGING BOLTED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS THE MINIMUM REQUIRED. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL BRIDGING REQUIRED FOR ERECTION. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
17. * INDICATES JOIST TO BE DOUBLE PITCHED TO CREATE ROOF RIDGE.
18. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED, SEE 6/S4.01.
19. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.



NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING AND T/STEEL ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. ←—→ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK. SEE 4/S5.01.
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR 15 KIP MAXIMUM.
6. ▶ INDICATES MOMENT CONNECTION. SEE 13 & 14/S5.01.
7. S.P. INDICATES SPLICE POINT. SEE 18/S5.01.
8. EXTEND AND ATTACH BOTTOM CHORD OF JOIST AT EACH COLUMN TO COLUMN, SEE DETAIL 7/S5.01
9. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
10. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
11. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
12. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
13. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
14. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
15. – – – – INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS THE MINIMUM REQUIRED.
16. ←—→ INDICATES DIRECTIONAL SPAN OF 3/4" FRT PLYWOOD SHEATHING, SEE 20/S5.04.
17. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED. SEE 6/S4.01.
18. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.

Joist Description	Quantity
G01	1
G02	1
M01	25
M02	2
M03	12
M04	12
M05	3
M06	5
M07	6
M08	4
M09	6
M23	1
M25	2
M26	2
M27	2
M28	2

1 S2.03 Framing Plan - Roof - Cafe. Wing
1/8" = 1'-0"

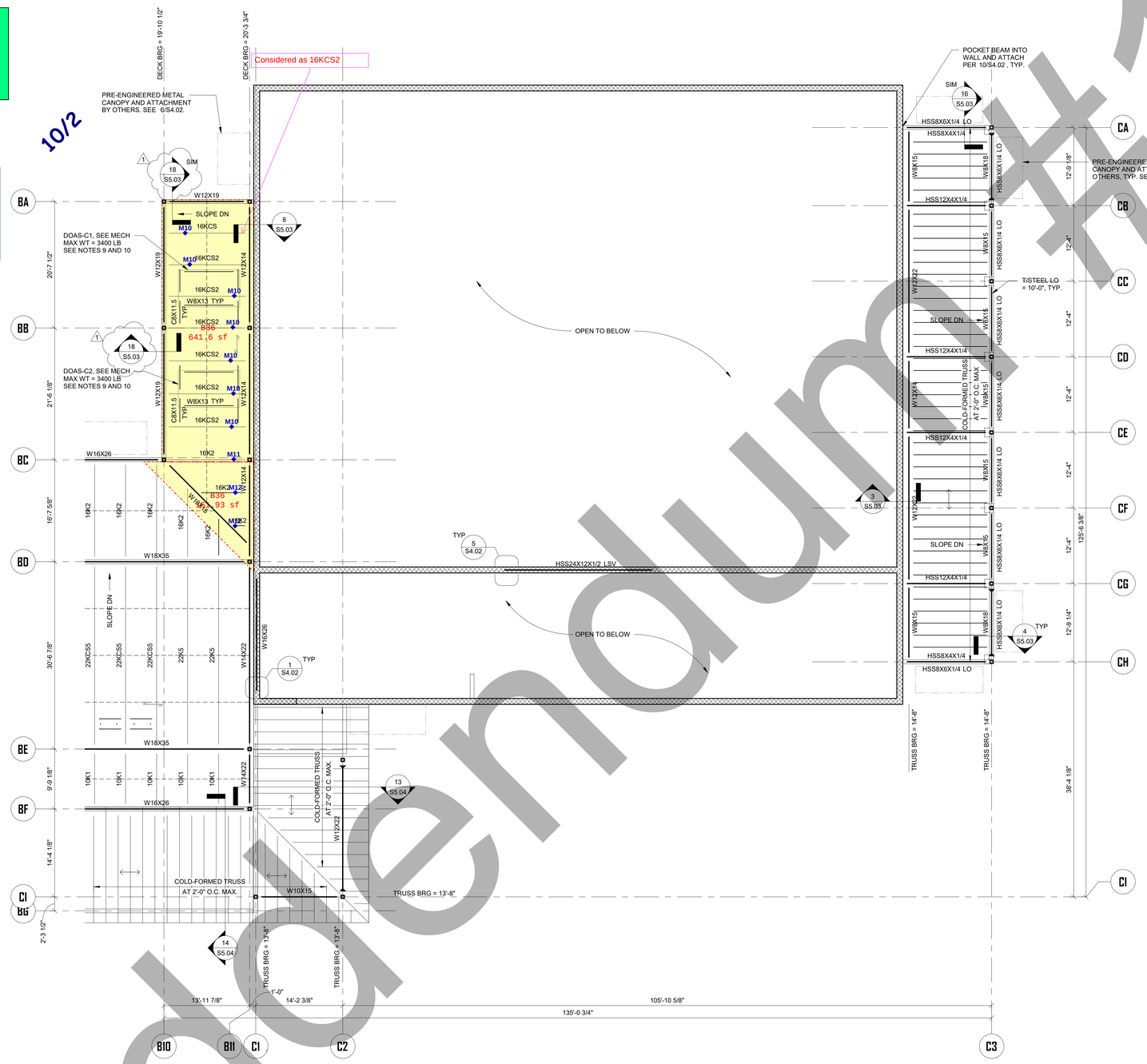


Qualifications:

OMD Deck section properties used
Roof deck quoted with galvanized G60
Loading due to RTU assumed to be included in KCS Joist Designation
Undesignated joist is considered as 16KCS2 Between grid B10 & C1

Exclusions:

Research & evaluation reports
Loading due to Angle kicker/braces
44LH13 SEE NOTE 17 S2.04b (Note number not clearly indicated)
Load from Basketball goal (load & location not found)
Load from wall



NOTES:

1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. ←→ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 4/S5.01.
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR **15** KIP MAXIMUM.
6. ▶ INDICATES MOMENT CONNECTION. SEE 13/S5.01 .
7. S.P. INDICATES SPLICE POINT. SEE 18/S5.01.
8. EXTEND AND ATTACH BOTTOM CHORD OF JOIST AT EACH COLUMN TO COLUMN. SEE DETAIL 7/S5.01
9. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
10. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
11. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
12. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
13. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
14. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
15. ←—←—→ INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS THE MINIMUM REQUIRED. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
16. ←—→ INDICATES DIRECTIONAL SPAN OF 3/4" FRT PLYWOOD SHEATHING, SEE 20/S5.04 .
17. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED. SEE 6/S4.01.
18. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.

Joist

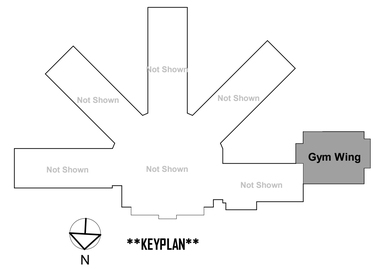
Description	Quantity
M10	7
M11	1
M12	2

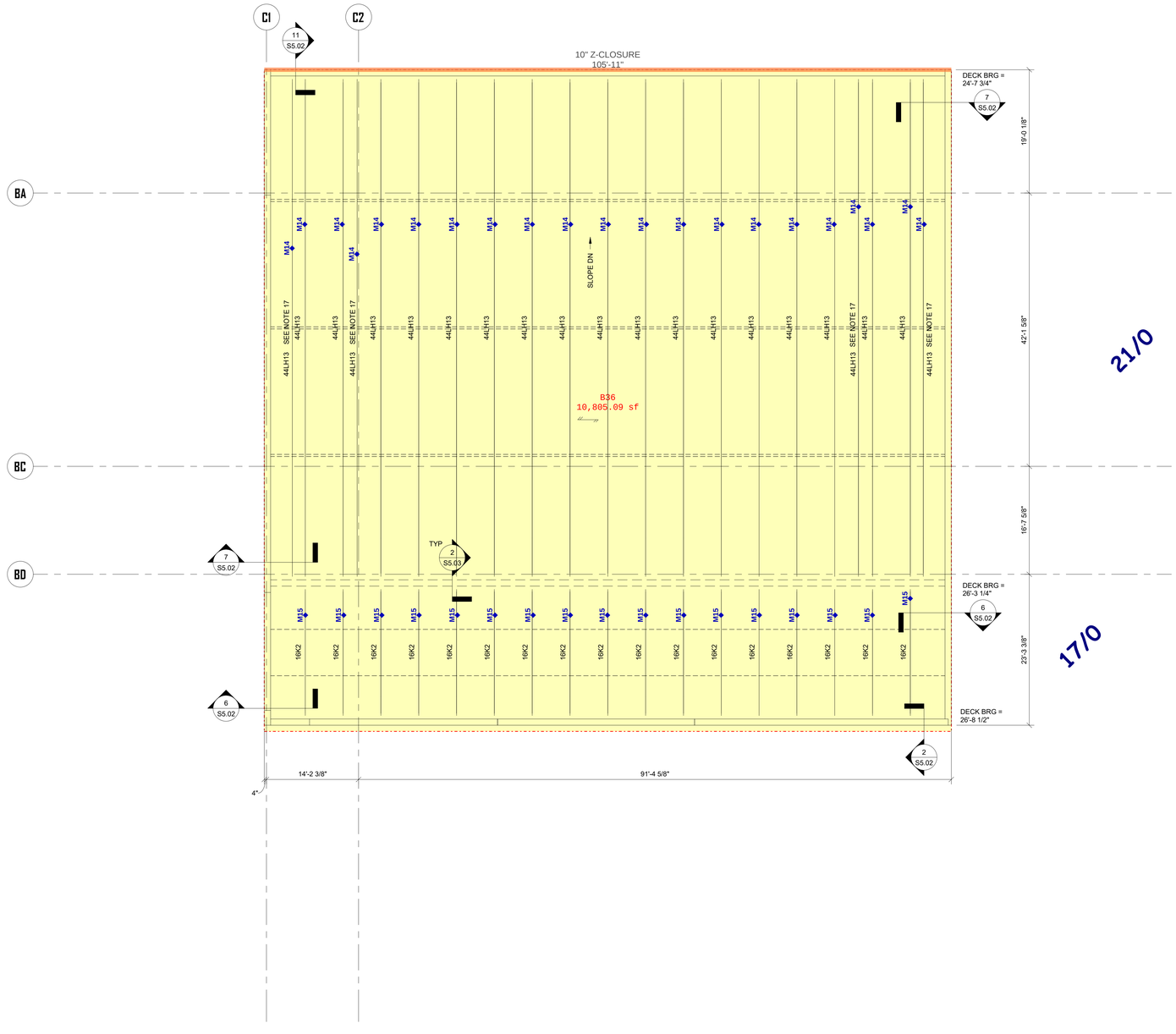
J=10/2

Deck V3 (ALL Page)										
	Description	Deck Type	Deck Gauge	Deck Finish	Lap or Waste	Deck SQ	No of Floors	Screw Pattern	Screw Total	Comments-Add'l Loads
	Roof	B36	20	G60	2%	16.18	1 Floor	12" O/C	538	
	Roof	B36	20	G60	2%	37.95	1 Floor	12" O/C	1,261	
	Roof	B36	20	G60	2%	981.58	1 Floor	12" O/C	32,723	

Deck Accessories (ALL Page)					
	Label	Description	Total Length	Total Qtys	Comments-Add'l Loads
	8" Z-CLOSURE	Other Accessories	565.86	570.00	Linear Feet, REF. 9/S5.02
	10" Z-CLOSURE	Other Accessories	105.92	110.00	Linear Feet, REF. 11/S5.02

1 S2.04a Framing Plan - Roof - Gym Wing
1/8" = 1'-0"





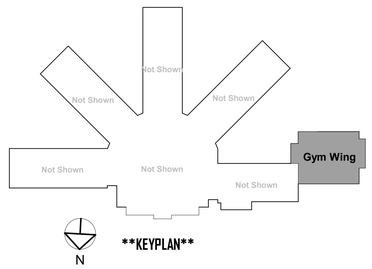
NOTES:

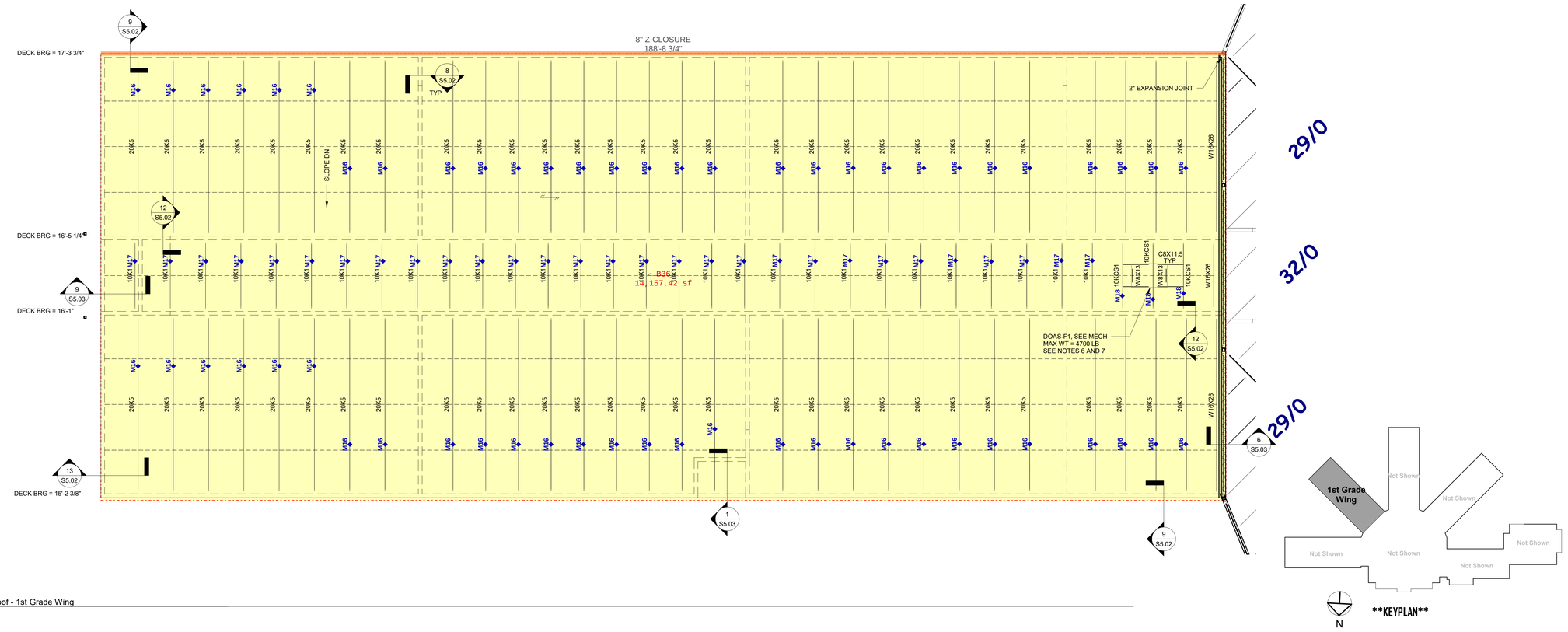
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. ⟵ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 4/S5.01.
5. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
6. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
7. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
8. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
9. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
10. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
11. ⟵⟵ INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS THE MINIMUM REQUIRED. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
12. ⟵=⟵ INDICATES CONT. VERT "X" RIGID BRIDGING BOLTED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS THE MINIMUM REQUIRED. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL BRIDGING REQUIRED FOR ERECTION. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
13. PROVIDE ADDITIONAL JOIST AT BASKETBALL GOAL AND ATTACHMENT POINTS. COORDINATE LOCATION OF BASKETBALL GOAL WITH ARCH.
14. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED, SEE 6/S4.01.
15. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.

Joist

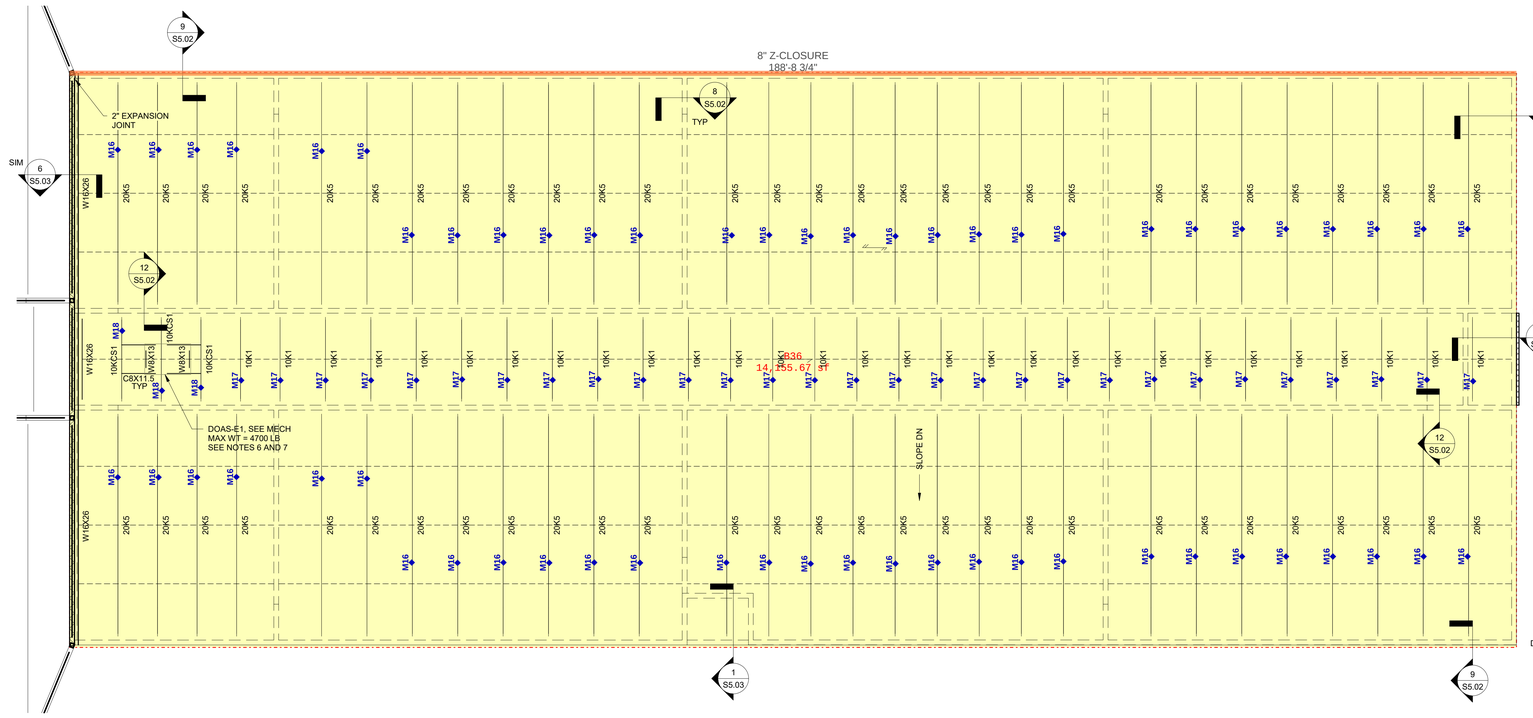
Description	Quantity
M14	21
M15	17

1 S2.04b Framing Plan - Roof - Gym Wing (High)
1/8" = 1'-0"





2 S2.05 Framing Plan - Roof - 1st Grade Wing
1/8" = 1'-0"

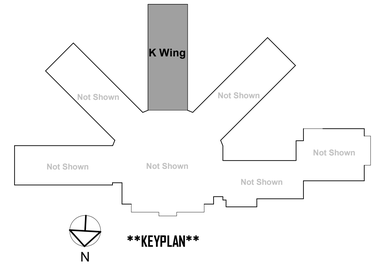


1 S2.05 Framing Plan - Roof - K Wing
1/8" = 1'-0"

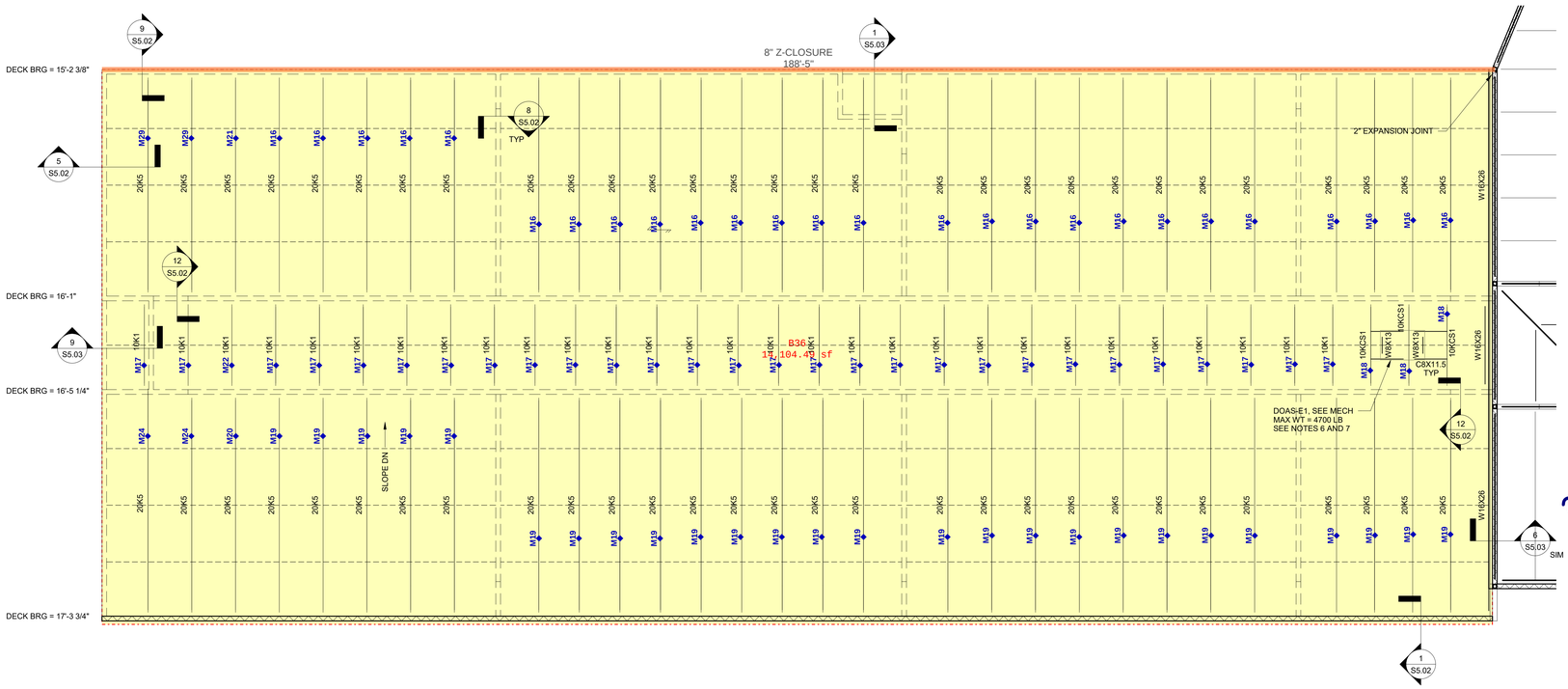
J=180/0

Joist		
Description	Quantity	
M16	116	
M17	58	
M18	6	

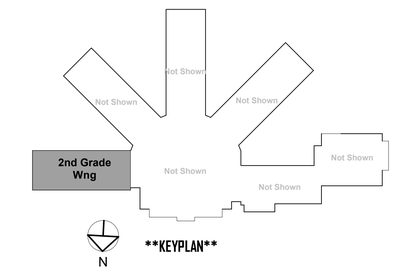
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION @ RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. ←—→ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 4/S5.01
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR **10** KIP MAXIMUM.
6. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
7. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
8. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
9. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
10. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
11. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
12. – – – – INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS MINIMUM REQUIRED. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
13. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED, SEE 6/S4.01.
14. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.



NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. <—→ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK. SEE 4/S5.01.
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR **10** KIP MAXIMUM.
6. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
7. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
8. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
9. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
10. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
11. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
12. – – – – INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS MINIMUM REQUIRED. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
13. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED. SEE 6/S4.01.
14. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.

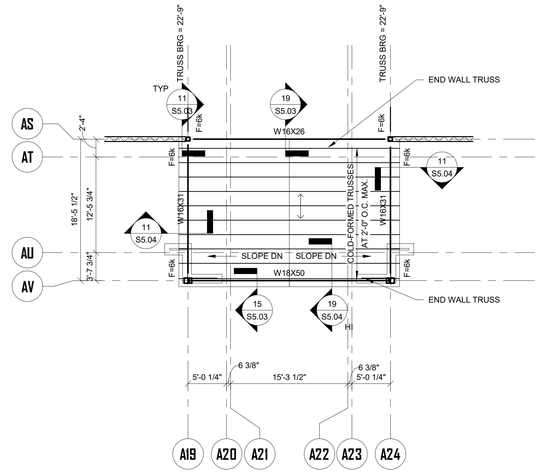


Joist Description	Quantity
M16	26
M17	28
M18	3
M19	26
M20	1
M21	1
M22	1
M24	2
M29	2

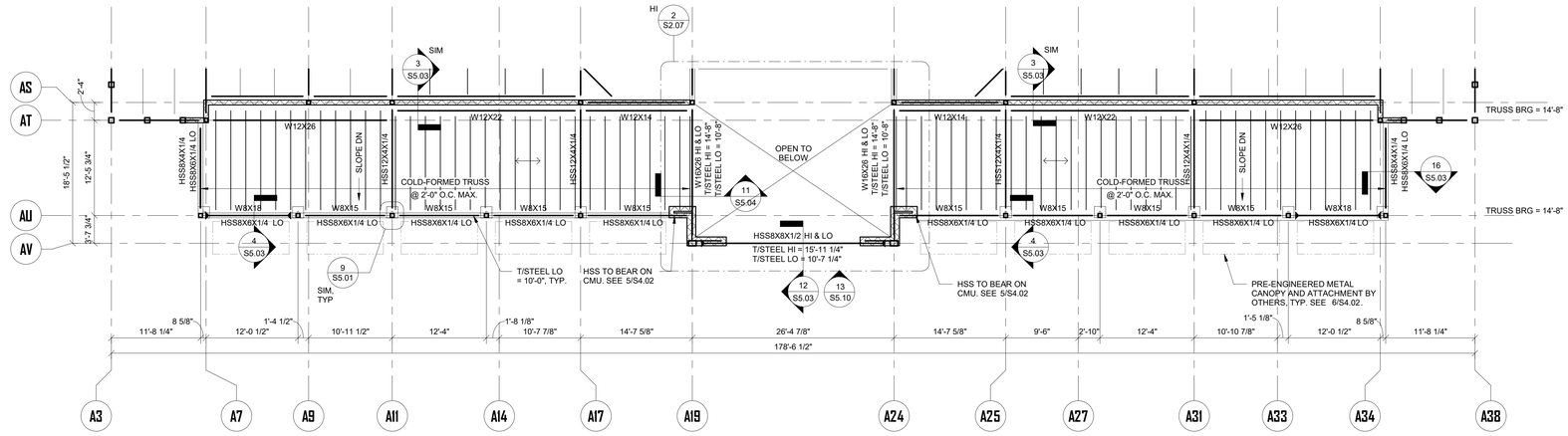


KEYPLAN

1 S2.06 Framing Plan - Roof - 2nd Grade Wing
1/8" = 1'-0"

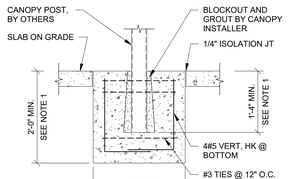


2 S2.07 Framing Plan - Roof - Enlarged Front Drop Off Area (High)
1/8" = 1'-0"



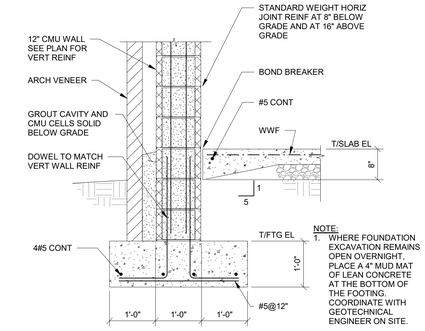
1 S2.07 Framing Plan - Roof - Enlarged Front Drop Off Area
1/8" = 1'-0"

NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. <⟷> INDICATES DIRECTIONAL SPAN OF 3/4" FRT PLYWOOD SHEATHING, SEE 20/S5.04.
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR **15** KIP MAXIMUM.
6. ▶ INDICATES MOMENT CONNECTION. SEE 13 & 14/S5.01.
7. EXTEND AND ATTACH BOTTOM CHORD OF JOIST AT EACH COLUMN TO COLUMN, SEE DETAIL 7/S5.01.
8. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
9. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
10. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED, SEE 6/S4.01.
11. F=x INDICATES MAXIMUM ALLOWABLE AXIAL FORCE REQUIRED IN BEAM CONNECTION. STEEL FABRICATOR TO DESIGN CONNECTION FOR AXIAL FORCE INDICATED.

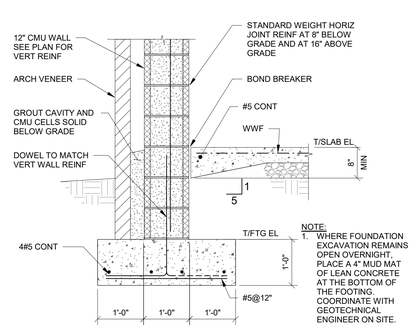


NOTES:
1. COORD WITH CANOPY MFR. INCREASE FOOTING DEPTH IF REQUIRED BY CANOPY MFR.
2. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

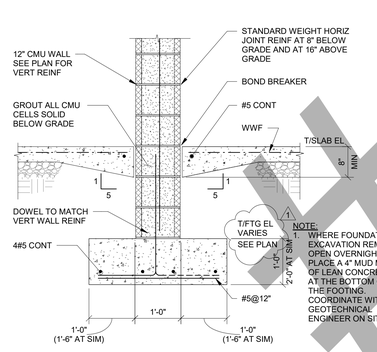
20 S3.01 TYPICAL STEEL PRE-ENGINEERED CANOPY COLUMN FOOTING
3/4" = 1'-0"



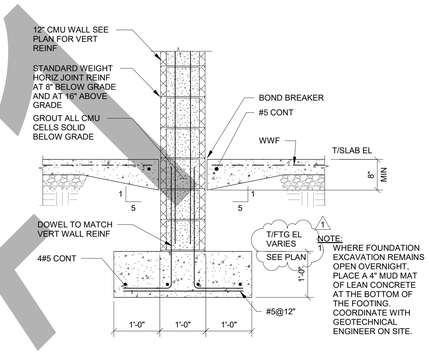
16 S3.01 TYPICAL EXTERIOR 12" CMU WALL FOOTING (DOUBLE LAYER REINFORCING)
3/4" = 1'-0"



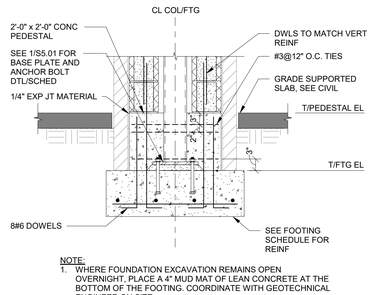
12 S3.01 TYPICAL EXTERIOR 12" CMU WALL FOOTING (SINGLE LAYER REINFORCING)
3/4" = 1'-0"



8 S3.01 TYPICAL INTERIOR 12" CMU WALL FOOTING (SINGLE LAYER REINFORCING)
3/4" = 1'-0"

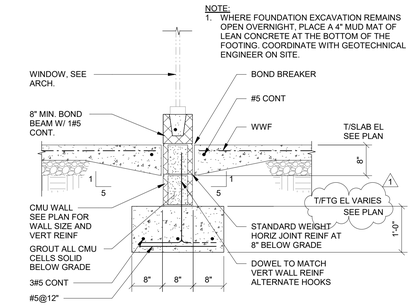


4 S3.01 TYPICAL INTERIOR 12" CMU WALL FOOTING (DOUBLE LAYER REINFORCING)
3/4" = 1'-0"

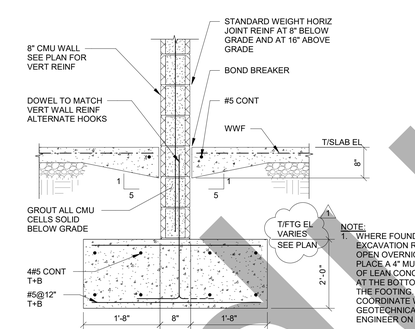


NOTE:
1. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

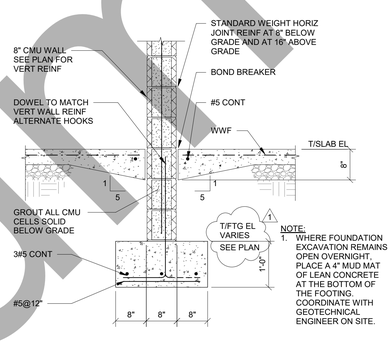
19 S3.01 TYPICAL COVERED WALK COLUMN FOOTING
3/4" = 1'-0"



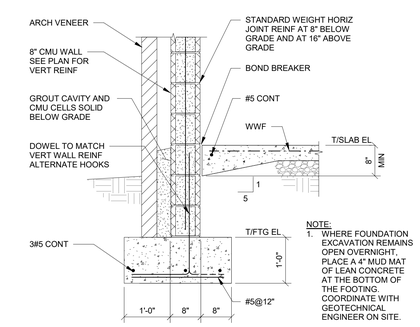
15 S3.01 SECTION AT RECEPTION/WAITING ROOM WALL
3/4" = 1'-0"



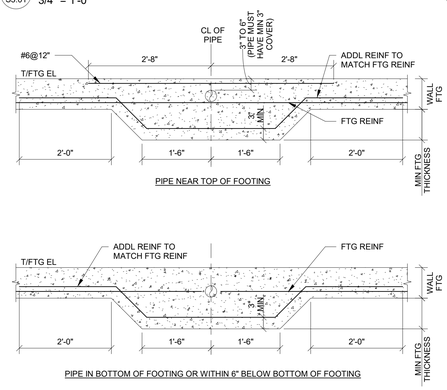
11 S3.01 TYPICAL INTERIOR 8" CMU SHEAR WALL FOOTING
3/4" = 1'-0"



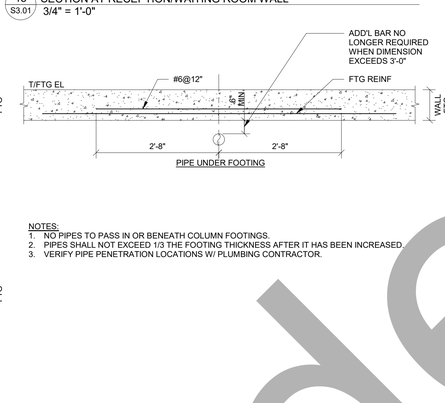
7 S3.01 TYPICAL INTERIOR 8" CMU WALL FOOTING
3/4" = 1'-0"



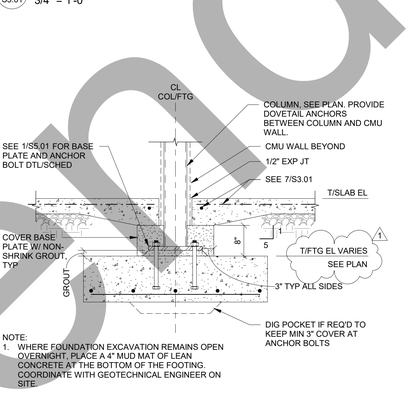
3 S3.01 TYPICAL EXTERIOR 8" CMU WALL FOOTING
3/4" = 1'-0"



18 S3.01 TYPICAL THICKENED FOOTING AT UNDERGROUND PIPING
3/4" = 1'-0"

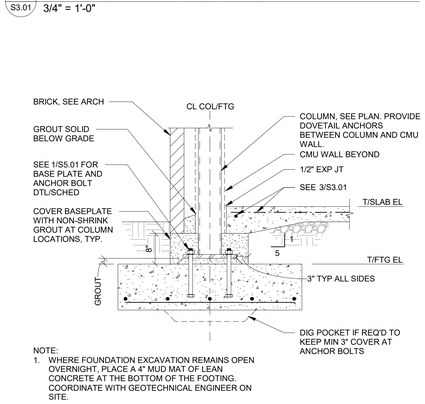


NOTES:
1. NO PIPES TO PASS IN OR BENEATH COLUMN FOOTINGS.
2. PIPES SHALL NOT EXCEED 1/3 THE FOOTING THICKNESS AFTER IT HAS BEEN INCREASED.
3. VERIFY PIPE PENETRATION LOCATIONS W/ PLUMBING CONTRACTOR.



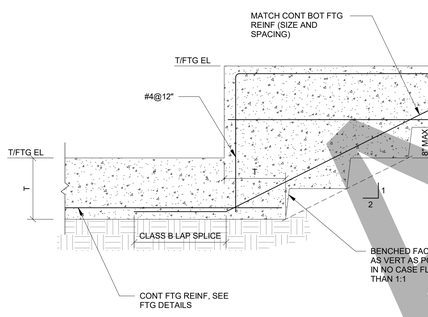
NOTE:
1. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

10 S3.01 TYPICAL INTERIOR STEEL COLUMN FOOTING WITHIN CMU WALL
3/4" = 1'-0"

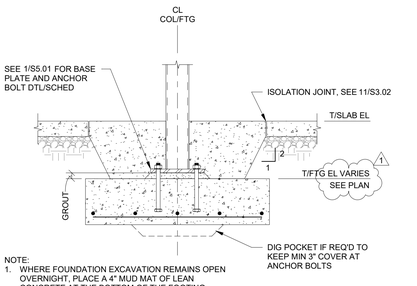


NOTE:
1. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

6 S3.01 TYPICAL EXTERIOR STEEL COLUMN FOOTING WITHIN CMU WALL
3/4" = 1'-0"

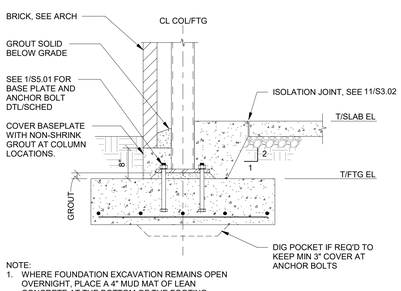


17 S3.01 TYPICAL CONTINUOUS STEPPED FOOTING
3/4" = 1'-0"



NOTE:
1. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

9 S3.01 TYPICAL INTERIOR STEEL COLUMN FOOTING
3/4" = 1'-0"



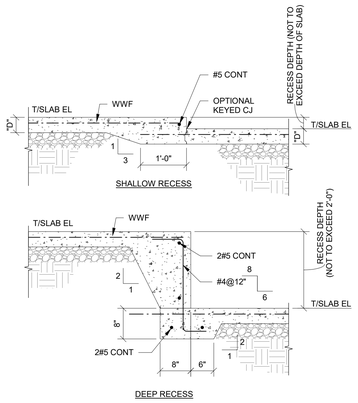
NOTE:
1. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

5 S3.01 TYPICAL EXTERIOR STEEL COLUMN FOOTING
3/4" = 1'-0"

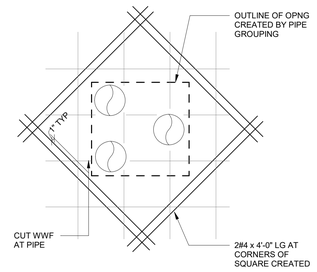
FOOTING SCHEDULE

MARK	SIZE (WxLxT)	REINF	REMARKS
F3.0	3'-0"x3'-0"x12"	3#5 EW	BOTTOM
F4.0	4'-0"x4'-0"x12"	4#5 EW	BOTTOM
F4.5	4'-6"x4'-6"x12"	4#5 EW	BOTTOM
F5.0	5'-0"x5'-0"x12"	5#5 EW	BOTTOM
F5.0X	5'-0"x5'-0"x12"	5#5 EW	TOP & BOTTOM
F6.0	6'-0"x6'-0"x15"	5#6 EW	TOP & BOTTOM
F10.0	10'-0"x10'-0"x18"	10#6 EW	TOP & BOTTOM
F15.0	15'-0"x10'-0"x18"	10#6 LW, 15#6 SW	TOP & BOTTOM

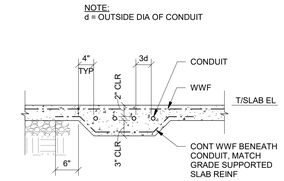
1 S3.01 FOOTING SCHEDULE
3/4" = 1'-0"



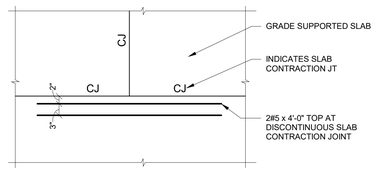
12 S3.02 TYPICAL GRADE SUPPORTED SLAB AT RECESS
3/4" = 1'-0"



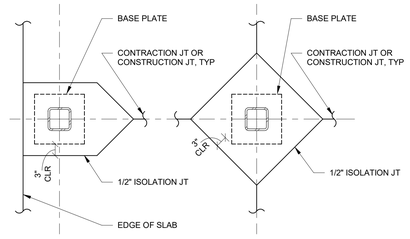
8 S3.02 TYPICAL SLAB REINFORCEMENT AT PIPING GROUPINGS (3 OR MORE)
3/4" = 1'-0"



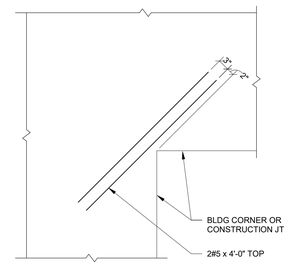
4 S3.02 TYPICAL GRADE SUPPORTED SLAB AT CONDUIT
3/4" = 1'-0"



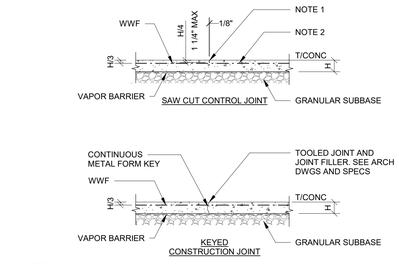
15 S3.02 TYPICAL REINFORCEMENT AT DISCONTINUOUS CONTRACTION JOINT
3/4" = 1'-0"



11 S3.02 TYPICAL ISOLATION JOINT AT STEEL COLUMN
3/4" = 1'-0"

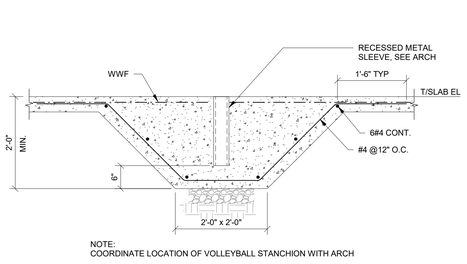


7 S3.02 TYPICAL REINFORCEMENT AT SLAB RE-ENTRANT CORNER
3/4" = 1'-0"



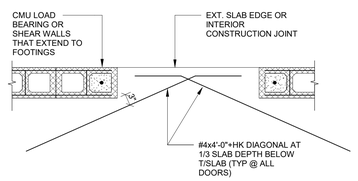
NOTE:
1. SAW CUT SHALL BE MADE SOON ENOUGH TO PREVENT CRACKING, BUT NOT SO SOON AS TO CAUSE SPALLING OF THE CONCRETE WHILE SAWING. FOR JOINT FILLER, SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. FOR JOINT SPACING, SEE STRUCTURAL SPECIFICATIONS.
2. SAW CUT OR DISCONTINUE REINF AT CONSTRUCTION JT.
3. KEYED CONSTRUCTION JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING.
4. WHERE REQUIRED FOR SUCCESSFUL INSTALLATION OF FLOOR FINISH SLAB SHALL BE WET CURED AND BE PLACED A MINIMUM OF 90 DAYS PRIOR TO INSTALLATION OF FLOOR FINISHES. AT CONTRACTOR'S OPTION DIAMOND DOWELS MAY BE USED IN LIEU OF CONTINUOUS CONCRETE KEY AT KEYED JOINT.
5. PROVIDE SLAB BOLSTERS A MAX 2'-0" APART FOR WWF SUPPORT

3 S3.02 TYPICAL GRADE SUPPORTED SLAB AT JOINTS
3/4" = 1'-0"

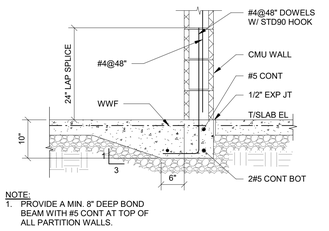


NOTE:
COORDINATE LOCATION OF VOLLEYBALL STANCHION WITH ARCH

14 S3.02 VOLLEYBALL SLEEVE FOOTING
3/4" = 1'-0"

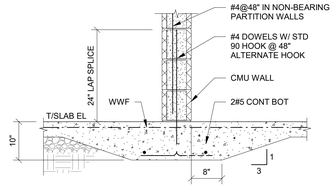


10 S3.02 TYPICAL RE-ENTRANT BARS AT CMU WALLS
3/4" = 1'-0"



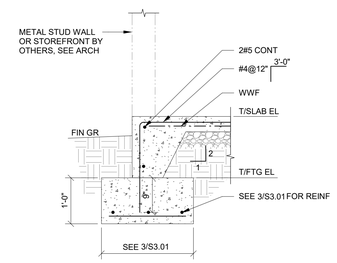
NOTE:
1. PROVIDE A MIN. 8" DEEP BOND BEAM WITH #5 CONT AT TOP OF ALL PARTITION WALLS.

6 S3.02 TYPICAL GRADE SUPPORTED SLAB AT NON-BEARING CMU WALL WITH CJ
3/4" = 1'-0"

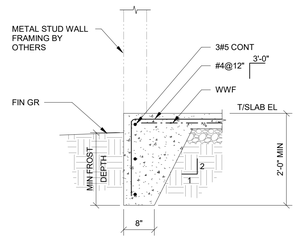


NOTE:
1. PROVIDE A MIN. 8" DEEP BOND BEAM WITH #5 CONT AT TOP OF ALL PARTITION WALLS.

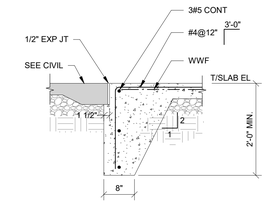
2 S3.02 TYPICAL GRADE SUPPORTED SLAB AT NON-BEARING CMU WALL
3/4" = 1'-0"



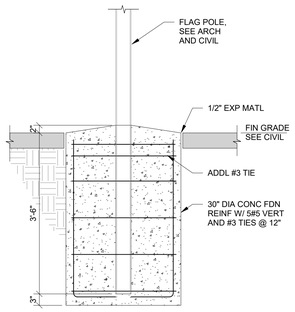
9 S3.02 TYPICAL TURNDOWN SLAB ON FOOTING
3/4" = 1'-0"



5 S3.02 TYPICAL TURNDOWN SLAB
3/4" = 1'-0"

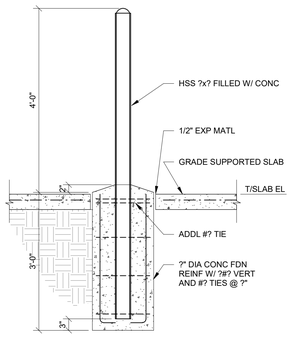


1 S3.02 TYPICAL TURNDOWN SLAB AT DOOR
3/4" = 1'-0"

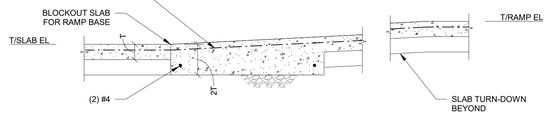


NOTE:
SEE CIVIL AND ARCH FOR LOCATION OF FLAG POLE.

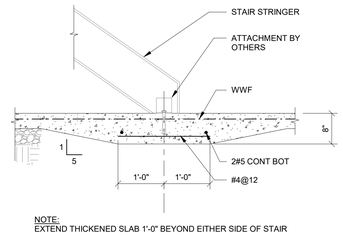
15 S3.03 FLAG POLE FOOTING DETAIL
3/4" = 1'-0"



11 S3.03 TYPICAL BOLLARD AT GRADE SUPPORTED SLAB
3/4" = 1'-0"

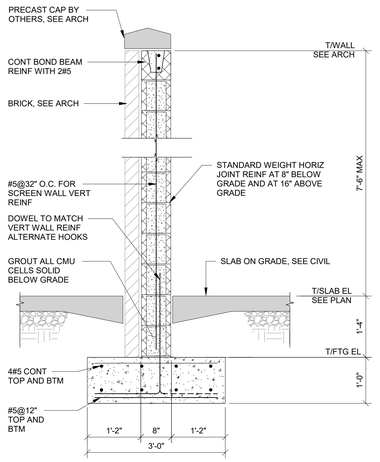


7 S3.03 RAMP DETAIL
3/4" = 1'-0"

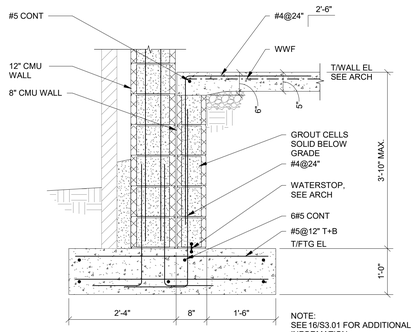


NOTE:
EXTEND THICKENED SLAB 1'-0" BEYOND EITHER SIDE OF STAIR

3 S3.03 TYPICAL THICKENED GRADE SUPPORTED SLAB AT STEEL STAIR
3/4" = 1'-0"

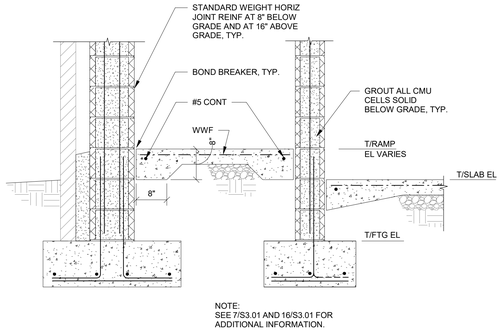


17 S3.03 SECTION AT SCREEN WALL
3/4" = 1'-0"



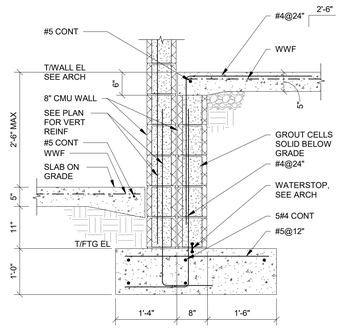
NOTE:
SEE 16/S3.01 FOR ADDITIONAL
INFORMATION.

13 S3.03 CMU RETAINING WALL AT BACK OF STAGE
3/4" = 1'-0"



NOTE:
SEE 7/S3.01 AND 16/S3.01 FOR
ADDITIONAL INFORMATION.

9 S3.03 SECTION AT RAMP BETWEEN CMU WALLS
3/4" = 1'-0"

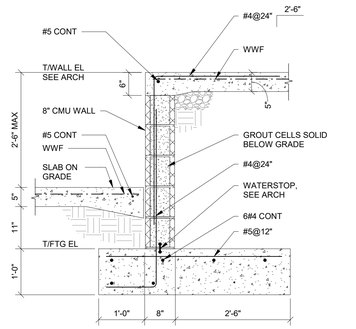


5 S3.03 CMU RETAINING WALL AT SIDE OF STAGE
3/4" = 1'-0"

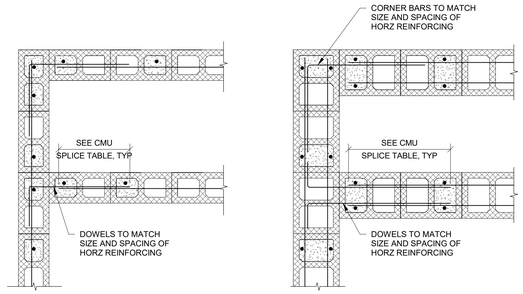
NOTE:
1. WHERE FOUNDATION EXCAVATION REMAINS OPEN OVERNIGHT, PLACE A 4" MUD MAT OF LEAN CONCRETE AT THE BOTTOM OF THE FOOTING. COORDINATE WITH GEOTECHNICAL ENGINEER ON SITE.

4 S3.03 EXTERIOR 8" CMU WALL AT CORNER OFFICE WINDOW
3/4" = 1'-0"

2 S3.03 EXTERIOR 12" CMU WALL AT CAFETERIA WINDOW
3/4" = 1'-0"



1 S3.03 CMU RETAINING WALL AT FRONT OF STAGE
3/4" = 1'-0"

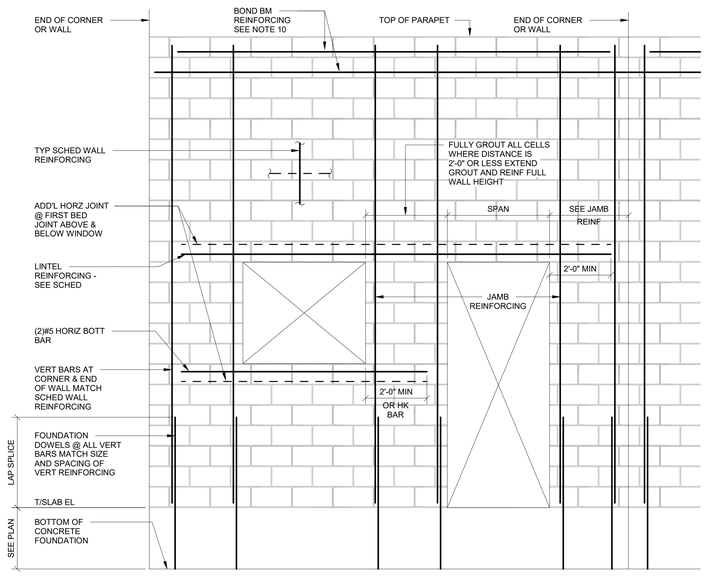


SINGLE LAYER REINFORCING

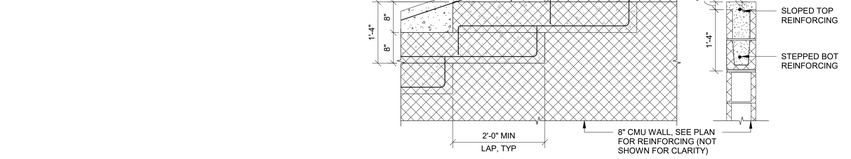
DOUBLE LAYER REINFORCING

19 / S4.01 TYPICAL MASONRY BOND BEAM REINFORCING DETAIL
3/4" = 1'-0"

NOTES:
1. ALL REINFORCING SHOWN IN ADDITION TO SCHEDULED WALL REINFORCEMENT.
2. ALL REINFORCING SHALL BE SOLID GROUTED USING 2500 PSI (SPEC COURSE GROUT).
3. REFER TO PLANS, GENERAL NOTES, AND WALL ELEVATIONS FOR TYPICAL VERTICAL AND HORIZONTAL WALL REINFORCEMENT.
4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF CONTROL JOINTS.
5. USE VERT BAR POSITIONERS AT 4 FT.
6. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAM BLOCKS EXCEPT JOINT REINFORCING.
7. HORIZONTAL REINFORCEMENT SHALL BE DISCONTINUOUS THROUGH CONTROL JOINTS EXCEPT BOND BEAMS AT ROOF, FLOORS AND AT MASONRY LINTELS.
8. PROVIDE HORIZONTAL JOINT REINFORCEMENT AT FIRST BED JOINT ABOVE AND BELOW OPENINGS AND EXTEND 1'-0" BEYOND THE OPENING.
9. COORDINATE LOCATION OF SOUND BLOCK IF SPECIFIED WITHIN REINFORCED CMU WALLS WITH ARCHITECTURAL INTERIOR ELEVATION DRAWINGS.
10. SEE 19/S4.01 FOR TYPICAL BOND BEAM REINFORCING DETAIL.



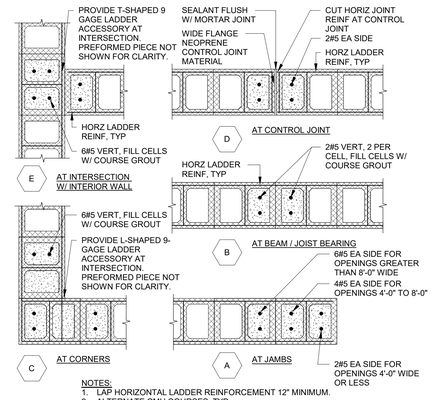
17 / S4.01 REINFORCEMENT MASONRY WALL ELEVATION
1/2" = 1'-0"



STANDARD LINTEL SCHEDULE		
WALL SIZE	LINTEL TYPE	REMARKS
OPENINGS UP TO 6'-0"		
4" BRICK	5/16"x5"xHORZ LEG BENT PL	ARCH TO PROVIDE HORZ DIM.
8" BLOCK	8"x8" U-BLOCK W/ 1#5 T&B	
12" BLOCK	12"x8" U-BLOCK W/ 2#5 T&B	
OPENINGS 6'-0" TO 8'-0"		
4" BRICK	3/8"x6"xHORZ LEG BENT PL	ARCH TO PROVIDE HORZ DIM.
8" BLOCK	8"x16" U-BLOCK W/ 1#6 T&B	
12" BLOCK	12"x16" U-BLOCK W/ 2#5 T&B	
OPENINGS 8'-0" TO 12'-0"		
8" BLOCK	8"x24" U-BLOCK W/ 1#6 T&B	
12" BLOCK	12"x16" U-BLOCK W/ 2#5 T&B	

NOTES:
1. ALL STEEL LINTELS TO BE GALVANIZED.
2. PROVIDE MINIMUM OF 6" BEARING FOR BRICK LINTELS.
3. SEE DETAIL 17/S4.01 FOR MINIMUM BEARING OF BLOCK LINTELS.
4. ALL OPENINGS ARE TO BE SHORED UNTIL THE MASONRY HAS CURED FOR A MINIMUM OF 72 HOURS.
5. SEE PLAN FOR STEEL LINTELS AT OPENINGS WIDER THAN 12'-0".

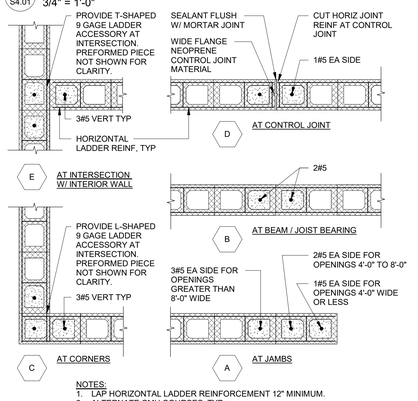
2 / S4.01 STANDARD LINTEL SCHEDULE
12" = 1'-0"



NOTES:
1. LAP HORIZONTAL LADDER REINFORCEMENT 12" MINIMUM.
2. ALTERNATE CMU COURSES, TYP.

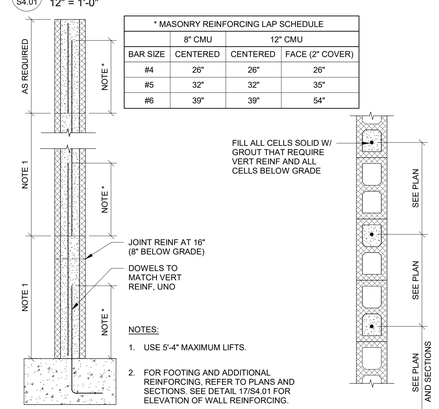
9 / S4.01 TYPICAL REINFORCING AT 12" LOAD BEARING CMU WALL
3/4" = 1'-0"

6 / S4.01 TYPICAL REINFORCING AT CMU WALL OPENING
3/4" = 1'-0"



NOTES:
1. LAP HORIZONTAL LADDER REINFORCEMENT 12" MINIMUM.
2. ALTERNATE CMU COURSES, TYP.

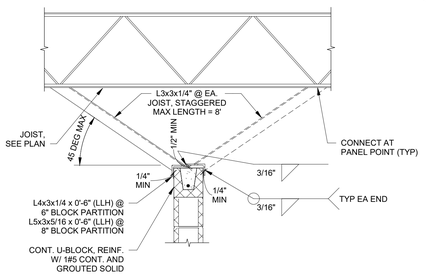
5 / S4.01 TYPICAL REINFORCING AT 8" LOAD BEARING CMU WALL
3/4" = 1'-0"



* MASONRY REINFORCING LAP SCHEDULE				
	8" CMU		12" CMU	
BAR SIZE	CENTERED	CENTERED	FACE (2" COVER)	
#4	26"	26"	26"	
#5	32"	32"	35"	
#6	39"	39"	54"	

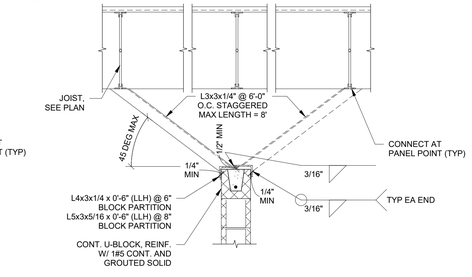
NOTES:
1. USE 5'-4" MAXIMUM LIFTS.
2. FOR FOOTING AND ADDITIONAL REINFORCING, REFER TO PLANS AND SECTIONS. SEE DETAIL 17/S4.01 FOR ELEVATION OF WALL REINFORCING.

1 / S4.01 TYPICAL REINFORCED CMU WALL
3/4" = 1'-0"



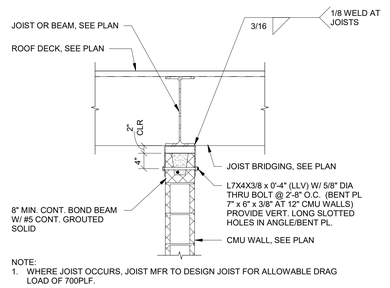
NOTES:
1. PROVIDE CONT. U-BLOCK REINF. W/ 1#5 CONT. GROUTED SOLID AT TOP OF ALL INTERIOR MASONRY PARTITION WALLS.
2. WHERE INTERIOR MASONRY PARTITION WALLS INTERSECT, PROVIDE CORNER BARS IN U-BLOCK.
3. WHERE THE HORIZONTAL UNBRACED LENGTH OF INTERIOR MASONRY PARTITION WALLS IS LESS THAN 10'-0", THE CONT. U-BLOCK AT TOP OF WALL WILL BE SUFFICIENT TO BRACE WALL. FOR HORIZONTAL UNBRACED LENGTHS GREATER THAN 10'-0", PROVIDE DIAGONAL BRACING AS SHOWN IN THIS DETAIL.

15 S4.02 TYPICAL BRACING OF PARTIAL HEIGHT INTERIOR NON-LOAD BEARING CMU WALLS (PERPENDICULAR)
3/4" = 1'-0"



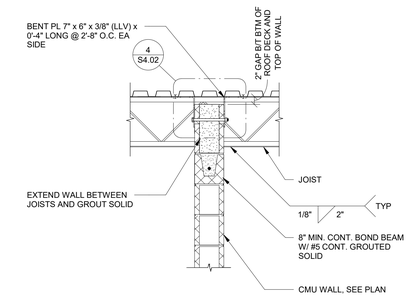
NOTES:
1. PROVIDE CONT. U-BLOCK REINF. W/ 1#5 CONT. GROUTED SOLID AT TOP OF ALL INTERIOR MASONRY PARTITION WALLS.
2. WHERE INTERIOR MASONRY PARTITION WALLS INTERSECT, PROVIDE CORNER BARS IN U-BLOCK.
3. WHERE THE HORIZONTAL UNBRACED LENGTH OF INTERIOR MASONRY PARTITION WALLS IS LESS THAN 10'-0", THE CONT. U-BLOCK AT TOP OF WALL WILL BE SUFFICIENT TO BRACE WALL. FOR HORIZONTAL UNBRACED LENGTHS GREATER THAN 10'-0", PROVIDE DIAGONAL BRACING AS SHOWN IN THIS DETAIL.

11 S4.02 TYPICAL BRACING OF PARTIAL HEIGHT INTERIOR NON-LOAD BEARING CMU WALLS (PARALLEL)
3/4" = 1'-0"

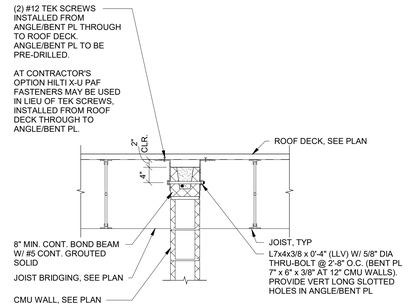


NOTE:
1. WHERE JOIST OCCURS, JOIST MFR TO DESIGN JOIST FOR ALLOWABLE DRAG LOAD OF 700PLF.

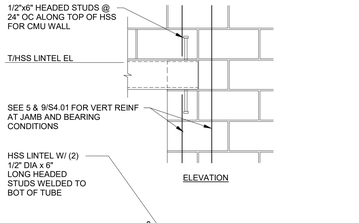
12 S4.02 TYPICAL CMU WALL BRACING AT INTERIOR CMU SHEAR WALL AT JOIST/BEAM
3/4" = 1'-0"



8 S4.02 TYPICAL CMU WALL BRACING AT INTERIOR CMU SHEAR WALL - JOIST PERPENDICULAR
3/4" = 1'-0"

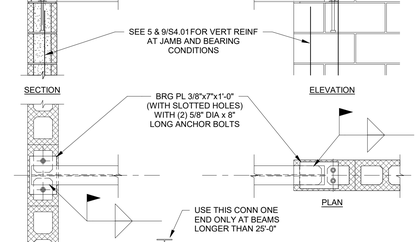


4 S4.02 TYPICAL CMU WALL BRACING AT INTERIOR CMU SHEAR WALL - JOIST PARALLEL
3/4" = 1'-0"



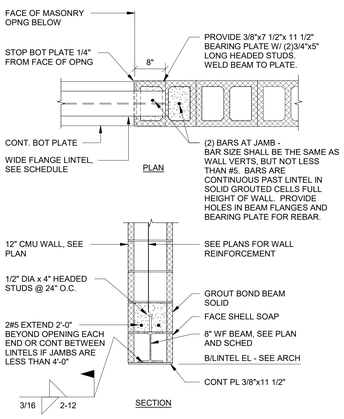
NOTES:
1. PROVIDE CMU WALL BRACING AT TOP OF WALL WHEN DISTANCE BETWEEN INTERSECTING WALLS AT BOTH ENDS EXCEED 22 TIMES THE WALL THICKNESS. LOCATION AND INSTALLATION OF BRACING FOR INTERIOR WALLS SHALL BE PER THE GOVERNING BUILDING CODE AND THE ARCHITECT'S DIRECTION. INTERSECTING WALLS MUST BE ANCHORED OR BONDED TOGETHER.
2. BETWEEN JOISTS, EXTEND CMU UP TO DECK.

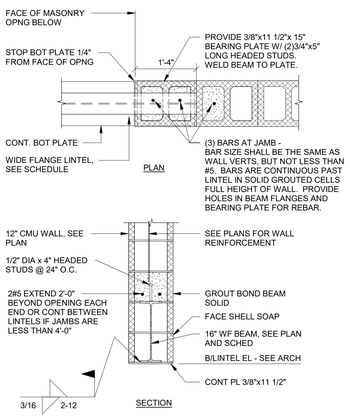
7 S4.02 TYPICAL CMU WALL BRACING AT INTERIOR CMU WALL AT JOIST
3/4" = 1'-0"



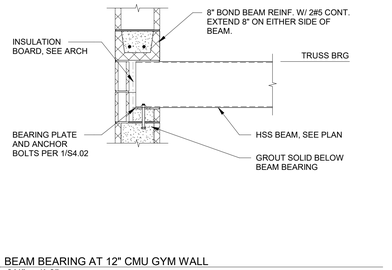
NOTES:
1. PROVIDE CMU WALL BRACING AT TOP OF WALL WHEN DISTANCE BETWEEN INTERSECTING WALLS AT BOTH ENDS EXCEED 22 TIMES THE WALL THICKNESS. LOCATION AND INSTALLATION OF BRACING FOR INTERIOR WALLS SHALL BE PER THE GOVERNING BUILDING CODE AND THE ARCHITECT'S DIRECTION. INTERSECTING WALLS MUST BE ANCHORED OR BONDED TOGETHER.
2. ANGLE BRACING REQUIRED AT EVERY OTHER JOIST.

3 S4.02 TYPICAL CMU WALL BRACING AT ROOF DECK
3/4" = 1'-0"

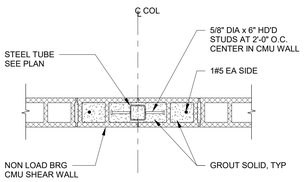




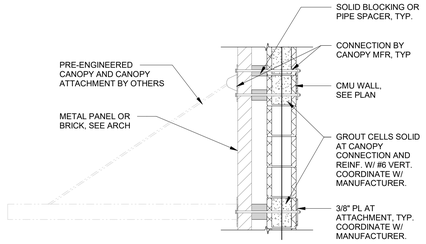
17 S4.02 TYPICAL WIDE FLANGE LINTEL
3/4" = 1'-0"



10 S4.02 BEAM BEARING AT 12" CMU GYM WALL
3/4" = 1'-0"



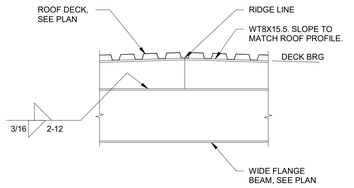
9 S4.02 TYPICAL COLUMN TO CMU SHEAR WALL
3/4" = 1'-0"



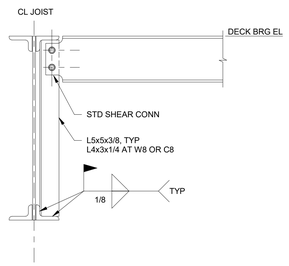
6 S4.02 TYPICAL SECTION WITH BRICK AT CANOPY
3/4" = 1'-0"

5 S4.02 TYPICAL CMU WALL AT HSS LINTEL BEARING
3/4" = 1'-0"

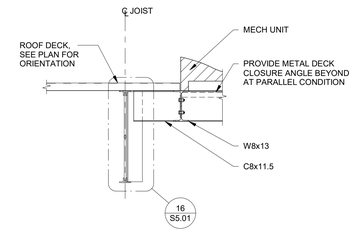
1 S4.02 TYPICAL CMU WALL AT BEAM BEARING
3/4" = 1'-0"



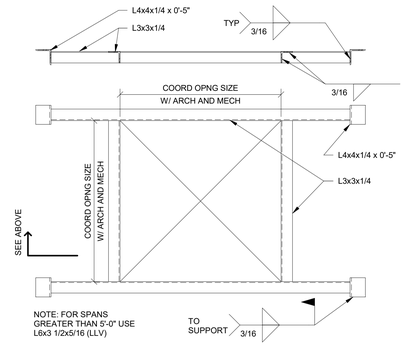
20 S5.01 SLOPED WT ON STEEL BEAM AT RIDGE
3/4" = 1'-0"



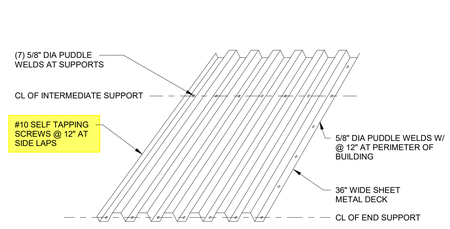
16 S5.01 TYPICAL BEAM TO JOIST CONNECTION
3/4" = 1'-0"



12 S5.01 TYPICAL ROOF DECK AT METAL MECHANICAL UNIT SUPPORT
3/4" = 1'-0"



8 S5.01 TYPICAL FRAMING AT ROOF DECK OPENING
3/4" = 1'-0"

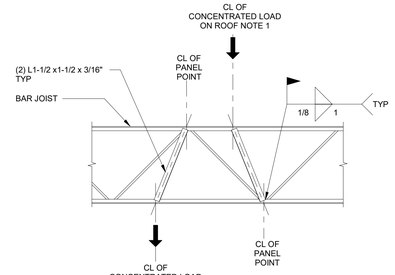


4 S5.01 TYPICAL ROOF DECK ATTACHMENT (36" WIDE SHEET)
3/4" = 1'-0"

TYPICAL JOIST TO SUPPORT MINIMUM WELD SCHEDULE			
JOIST SECTION	MINIMUM FILLET WELD AT EACH SIDE OF JOIST SEAT		REMARKS
	SIZE	LENGTH	
K1-12	1/8"	2 1/2"	
LH02-06	3/16"	2 1/2"	
LH07-17	1/4"	2 1/2"	
DLH10-17	1/4"	2 1/2"	
DLH18-25/JG	1/4"	4"	

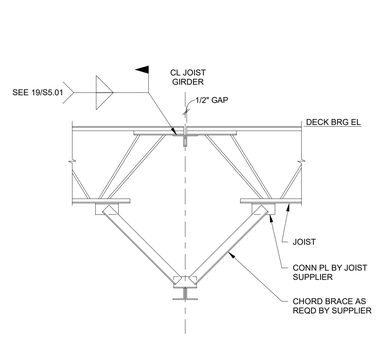
NOTES:
1. JOIST SECTION IS THE LAST DIGIT(S) OF JOIST DESIGNATION.
2. WELDS ARE SJI MINIMUM. JOIST DESIGNER TO CONFIRM WELDS ARE ADEQUATE FOR UPLIFT LOADS.

19 S5.01 TYPICAL JOIST TO SUPPORT MINIMUM WELD SCHEDULE
12" = 1'-0"

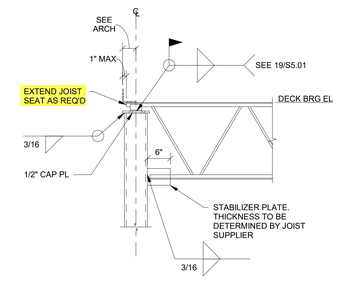


NOTES:
1. CONCENTRATED LOAD GREATER THAN 100 LBS BUT NOT TO EXCEED 300 LBS. MAX OF 2 LOADS TOTAL PER JOIST.
2. ANGLES ARE NOT REQUIRED WHERE CONCENTRATED LOAD IS WITHIN 6" OF PANEL POINT.

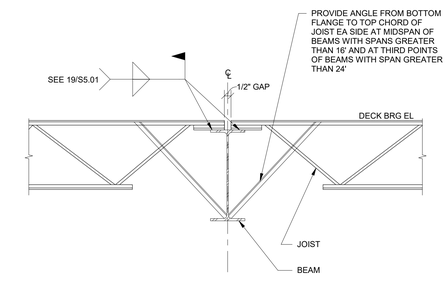
15 S5.01 TYPICAL JOIST REINFORCEMENT (CONCENTRATED LOAD)
3/4" = 1'-0"



11 S5.01 TYPICAL JOIST TO JOIST GIRDER CONNECTION
3/4" = 1'-0"

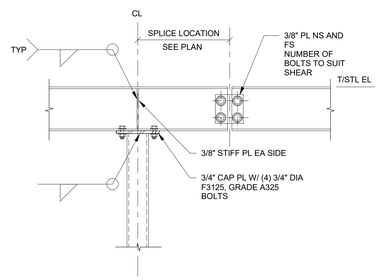


7 S5.01 TYPICAL JOIST TO END COLUMN CONNECTION
3/4" = 1'-0"

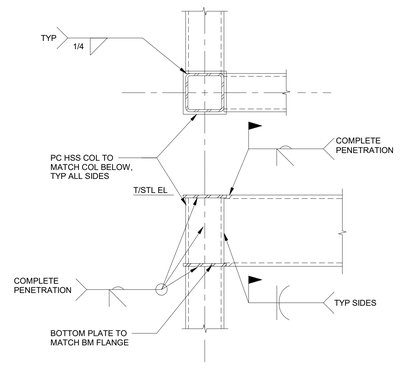


NOTES:
1. WHEN SUPPORT WIDTH DOES NOT MEET MIN SJI JOIST BEARING REQUIRED LAP JOISTS SIDE BY SIDE AT SUPPORT FOR MINIMUM BEARING.
2. WHERE JOISTS ARE 30% LONGER THAN JOISTS ON OPPOSITE SIDE, LAP JOISTS W/ FULL BEAM FLANGE BEARING.
3. 3 1/2" SEAT TO BE PROVIDED AT SLOPED CONDITIONS. MATCH SEAT DEPTH FOR JOIST ON EACH SIDE OF BEAM.

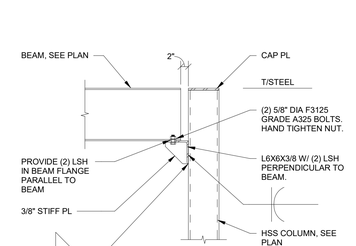
3 S5.01 TYPICAL JOIST TO BEAM INTERIOR CONNECTION
3/4" = 1'-0"



18 S5.01 TYPICAL CONTINUOUS BEAM SHEAR SPLICE
3/4" = 1'-0"



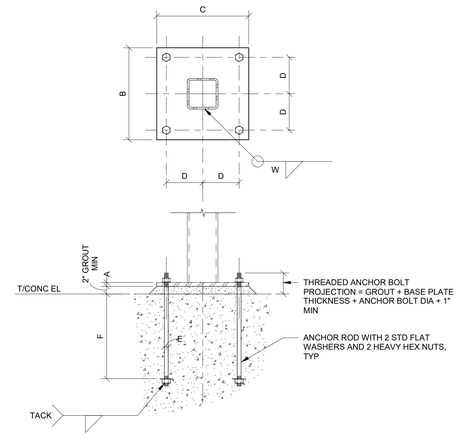
14 S5.01 TYPICAL TUBE COLUMN CONNECTIONS (MOMENT CONNECTION)
3/4" = 1'-0"



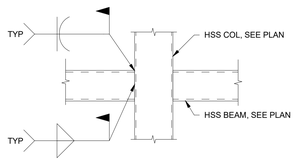
10 S5.01 BEAM TO COLUMN CONNECTION AT EXPANSION JOINT
3/4" = 1'-0"

BASE PLATE AND ANCHOR BOLT SCHEDULE								
MARK	BASE PLATE				ANCHOR BOLTS			WELD
	A	B	C	D	E	F	NO	W
BP1	3/4"	12"	12"	4 1/2"	3/4"	9"	4	1/4
BP2	1"	14"	14"	5 1/2"	1"	9"	4	1/4
BP3	1 1/4"	14"	18"	5" / 7"	1"	12"	4	1/4

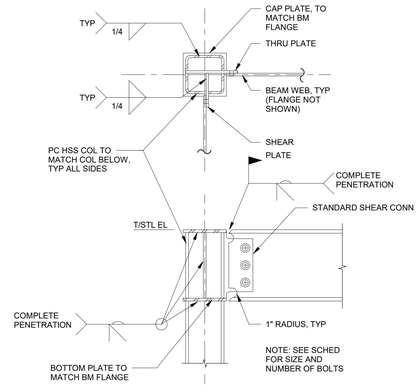
NOTE:
1. PROVIDE PLATE WASHERS AT ALL MOMENT FRAME COLUMNS AS INDICATED BELOW. WELD WASHERS ALL AROUND WITH 3/16" FILLET WELD.
   3/4" DIA ANCHORS: 2" x 2" x 1/4" THICK PLATE WASHER.
   1" DIA ANCHORS: 3" x 3" x 3/8" THICK PLATE WASHER.



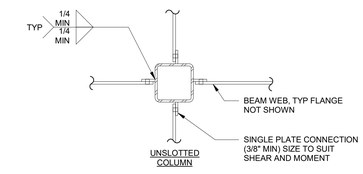
1 S5.01 TYPICAL BASE PLATE AND ANCHOR BOLT (TUBE)
3/4" = 1'-0"



17 S5.01 TYPICAL TUBE COLUMN CONNECTIONS (HSS BEAM)
3/4" = 1'-0"

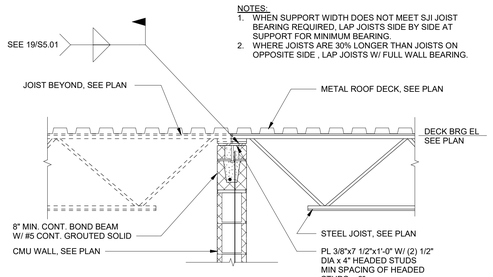


13 S5.01 TYPICAL TUBE COLUMN CONNECTIONS (MOMENT CONNECTION)
3/4" = 1'-0"

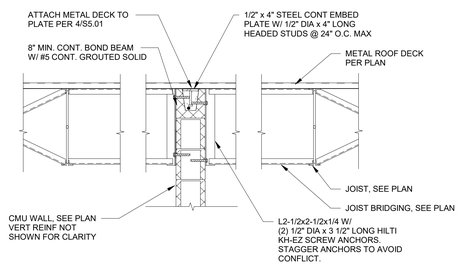


NOTE:
CLOSE COLUMN WITH 1/2" CAP PLATE, TYP

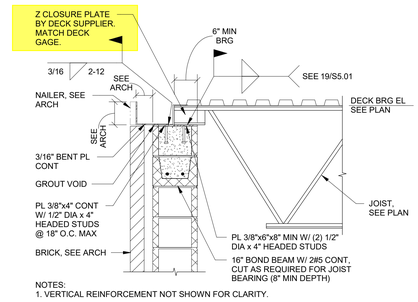
9 S5.01 TYPICAL TUBE COLUMN CONNECTIONS
3/4" = 1'-0"



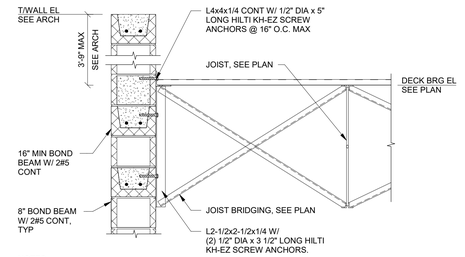
12 S5.02 TYPICAL JOIST BEARING ON INTERIOR 8" CMU WALL
3/4" = 1'-0"



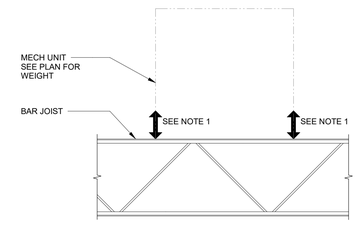
8 S5.02 CMU WALL SUPPORTING ROOF DECK
3/4" = 1'-0"



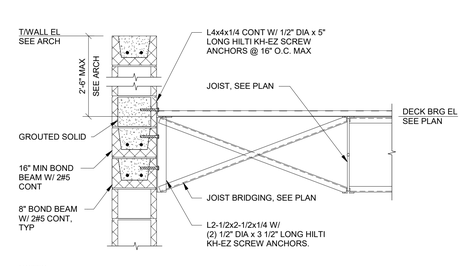
11 S5.02 TYPICAL 12" CMU WALL AT JOIST (PERPENDICULAR W/O PARAPET, LH-SERIES)
3/4" = 1'-0"



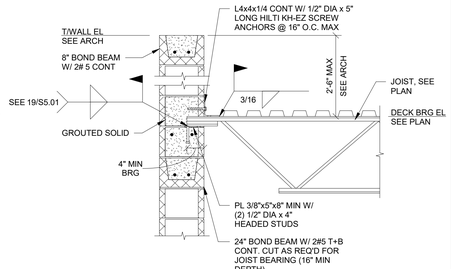
7 S5.02 TYPICAL 12" CMU WALL AT JOIST (PARALLEL, LH-SERIES)
3/4" = 1'-0"



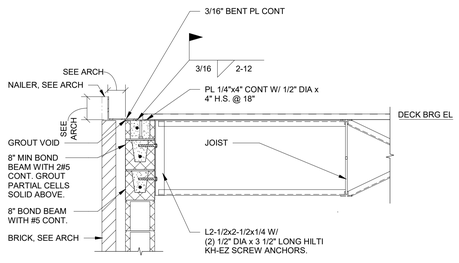
3 S5.02 JOIST WIND LOADS AT RTU
3/4" = 1'-0"



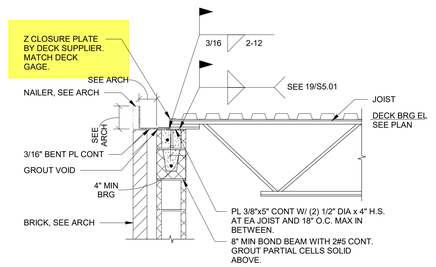
6 S5.02 TYPICAL 12" CMU WALL AT JOIST (PARALLEL, K-SERIES)
3/4" = 1'-0"



2 S5.02 TYPICAL 12" CMU WALL AT JOIST (PERPENDICULAR, K-SERIES)
3/4" = 1'-0"



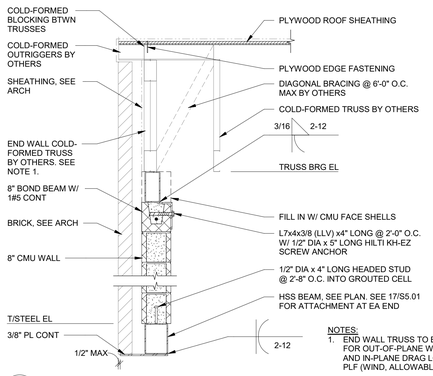
13 S5.02 TYPICAL 8" CMU WALL AT JOIST (PARALLEL W/O PARAPET)
3/4" = 1'-0"



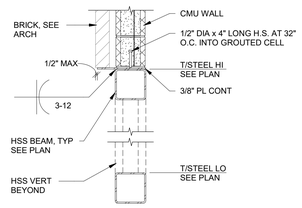
9 S5.02 TYPICAL 8" CMU WALL AT JOIST (PERPENDICULAR W/O PARAPET)
3/4" = 1'-0"

5 S5.02 TYPICAL 8" CMU WALL AT JOIST (PARALLEL)
3/4" = 1'-0"

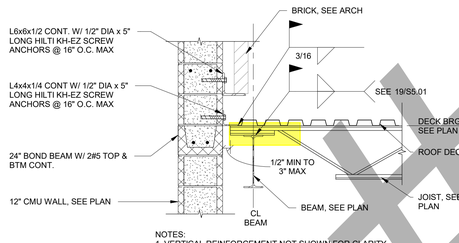
1 S5.02 TYPICAL 8" CMU WALL AT JOIST (PERPENDICULAR)
3/4" = 1'-0"



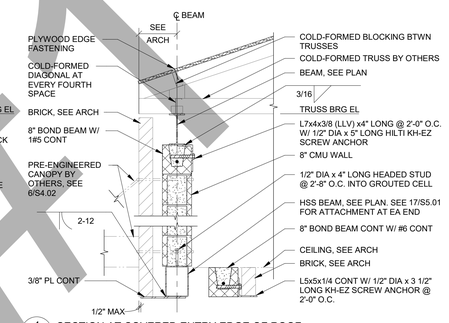
16 S5.03 SECTION AT SIDE EAVE OF COVERED ENTRY
3/4" = 1'-0"



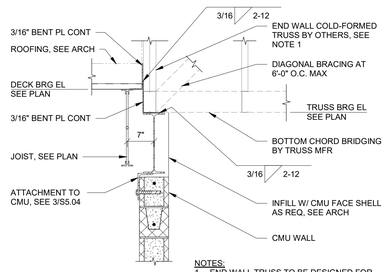
12 S5.03 SECTION AT FRONT GABLE END AT STEEL TRUSS FEATURE
3/4" = 1'-0"



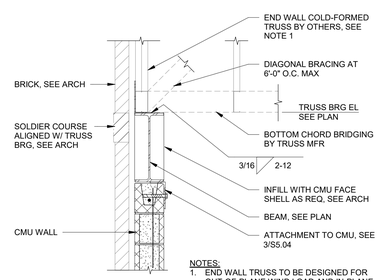
8 S5.03 SECTION AT LOW ROOF JOIST FRAMING AT GYM WALL
3/4" = 1'-0"



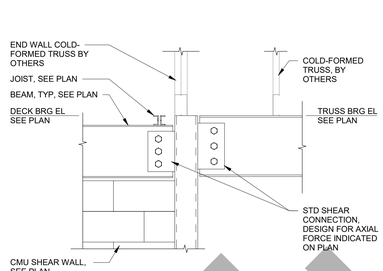
4 S5.03 SECTION AT COVERED ENTRY EDGE OF ROOF
3/4" = 1'-0"



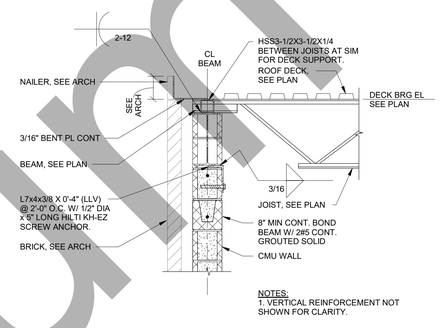
19 S5.03 SECTION AT GABLE ROOF TRANSITION AT JOIST
3/4" = 1'-0"



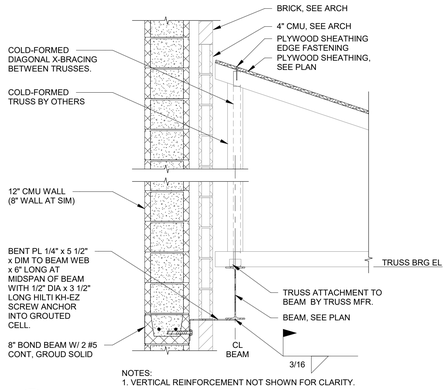
15 S5.03 SECTION AT FRONT GABLE END AT HIGH ROOF
3/4" = 1'-0"



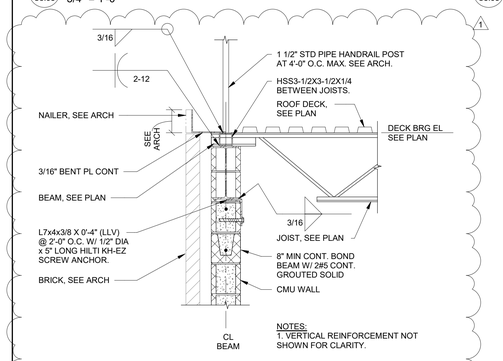
11 S5.03 SECTION AT GABLE ROOF TRANSITION AT COLUMN
3/4" = 1'-0"



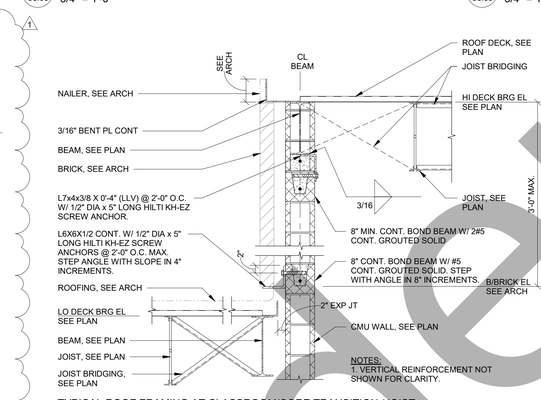
7 S5.03 TYPICAL EDGE OF ROOF AT STEEL BEAM WITH CMU WALL (JOIST PERPENDICULAR)
3/4" = 1'-0"



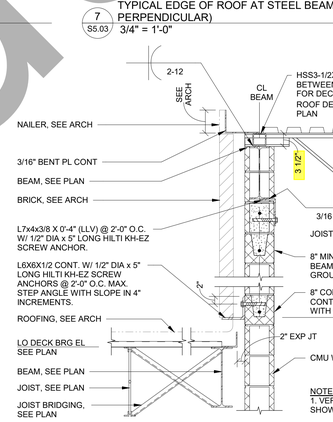
3 S5.03 FRAMING TO 12" CMU WALL AT GYM LOW ROOF
3/4" = 1'-0"



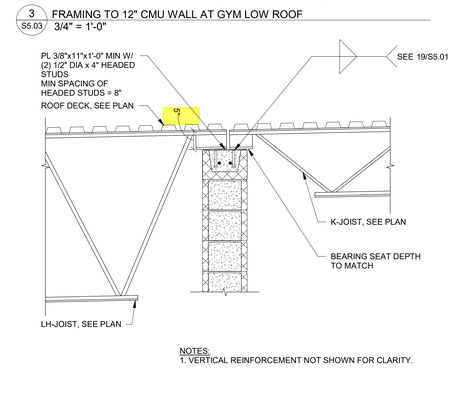
18 S5.03 TYPICAL EDGE OF ROOF AT STEEL BEAM WITH CMU WALL WITH HANDRAIL
3/4" = 1'-0"



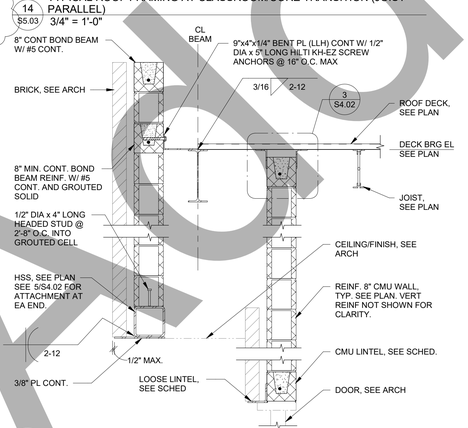
14 S5.03 TYPICAL ROOF FRAMING AT CLASSROOM/CORE TRANSITION (JOIST PARALLEL)
3/4" = 1'-0"



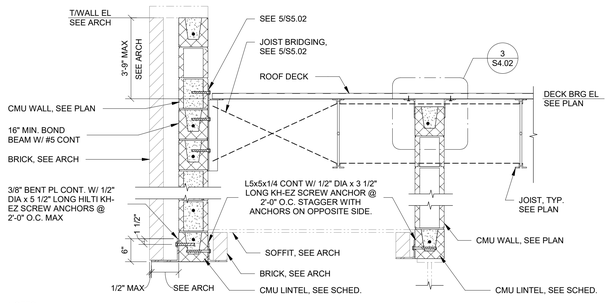
6 S5.03 TYPICAL ROOF FRAMING AT CLASSROOM/CORE TRANSITION (JOIST PERPENDICULAR)
3/4" = 1'-0"



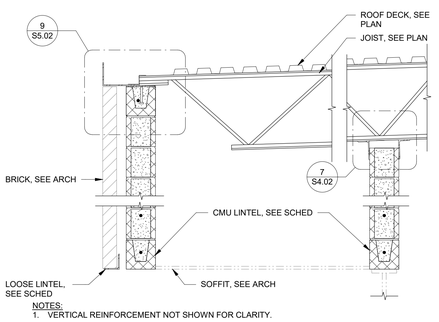
2 S5.03 JOIST BEARING AT GYM INTERIOR WALL
3/4" = 1'-0"



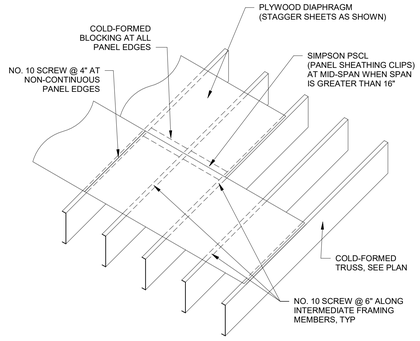
13 S5.03 SECTION AT ROOF FRAMING AT RECESSED STOOP
3/4" = 1'-0"



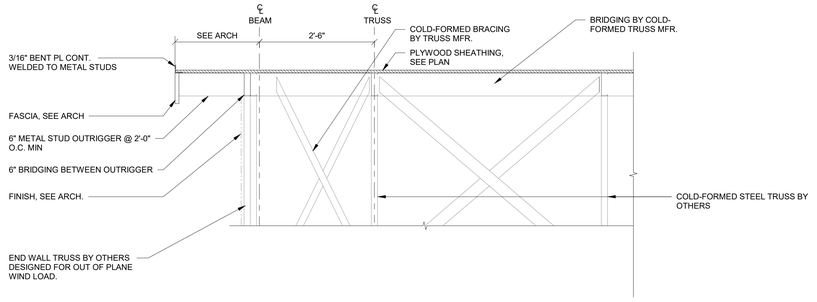
9 S5.03 PARALLEL ROOF JOIST AT CLASSROOM ENTRY
3/4" = 1'-0"



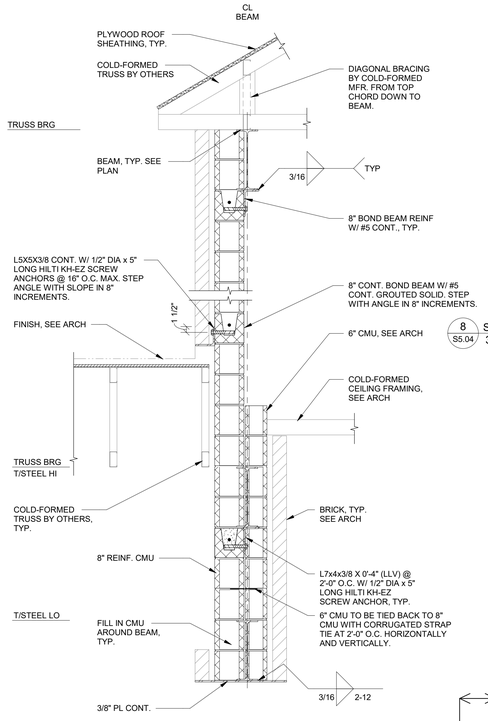
1 S5.03 PERPENDICULAR ROOF JOIST AT CLASSROOM ENTRY
3/4" = 1'-0"



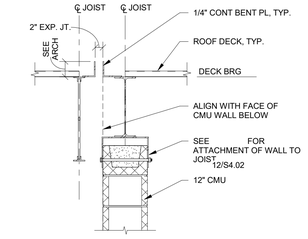
20 S5.04 PLYWOOD ROOF DIAPHRAGM DETAIL
3/4" = 1'-0"



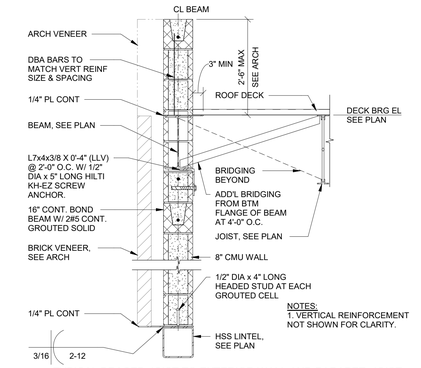
19 S5.04 TYPICAL END WALL TRUSS DETAIL
3/4" = 1'-0"



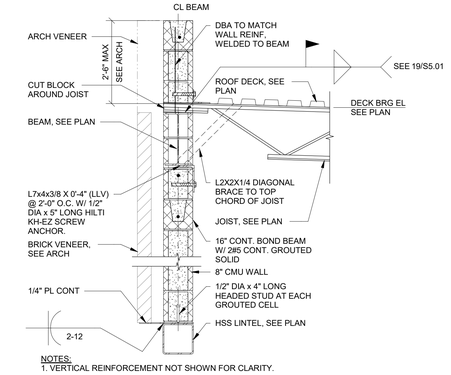
11 S5.04 SECTION AT FRONT ENTRY HIGH ROOF
3/4" = 1'-0"



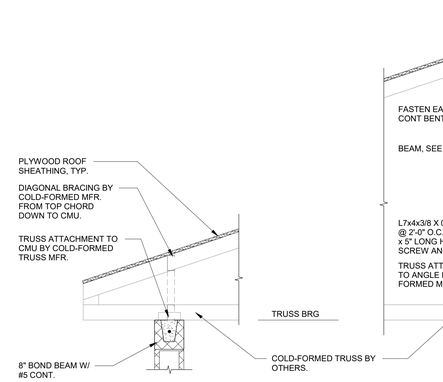
8 S5.04 SECTION AT ROOF DECK AT EXPANSION JOINT
3/4" = 1'-0"



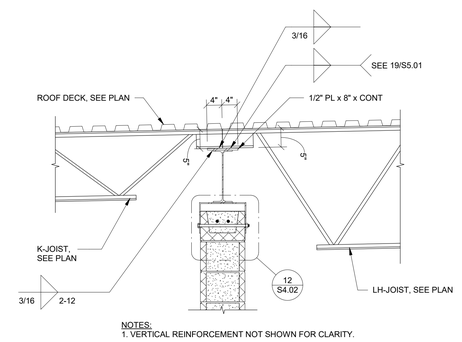
4 S5.04 TYPICAL BRACED JOIST TO EXTERIOR WALL AND PARAPET, JOIST PARALLEL
3/4" = 1'-0"



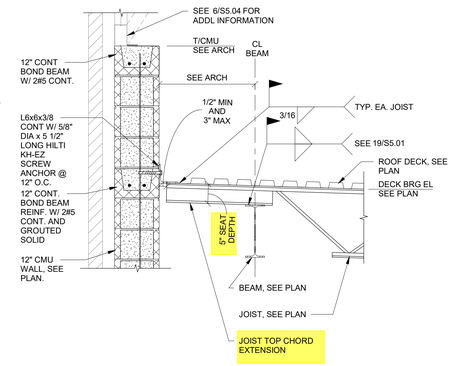
3 S5.04 TYPICAL BRACED JOIST TO EXTERIOR WALL AND PARAPET, JOIST PERPENDICULAR
3/4" = 1'-0"



14 S5.04 ROOF TRUSS OVER COVERED DELIVERY STOOP
3/4" = 1'-0"



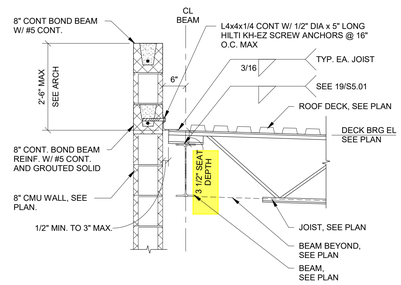
6 S5.04 SECTION AT CAFETERIA WINDOW PARAPET
3/4" = 1'-0"



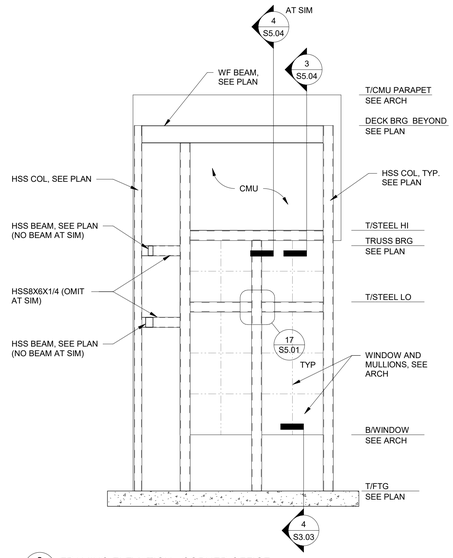
2 S5.04 JOIST EXTENSION TO CAFETERIA WALL
3/4" = 1'-0"

NOTES:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

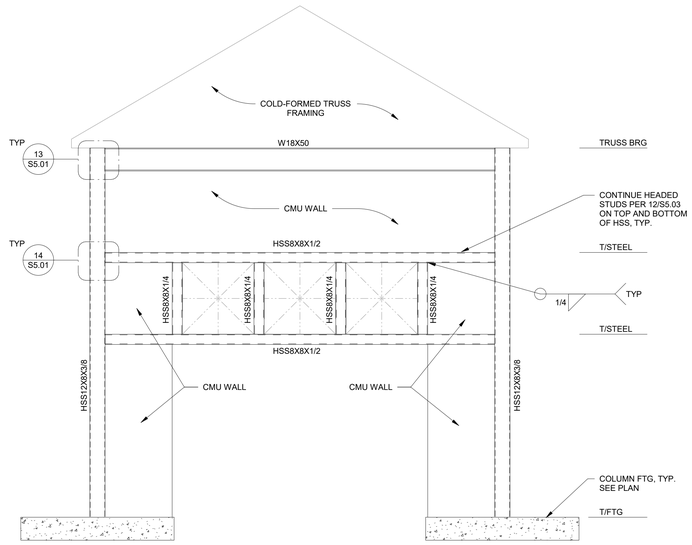
5 S5.04 JOIST BEARING TRANSITION AT COMMONS AREA
3/4" = 1'-0"



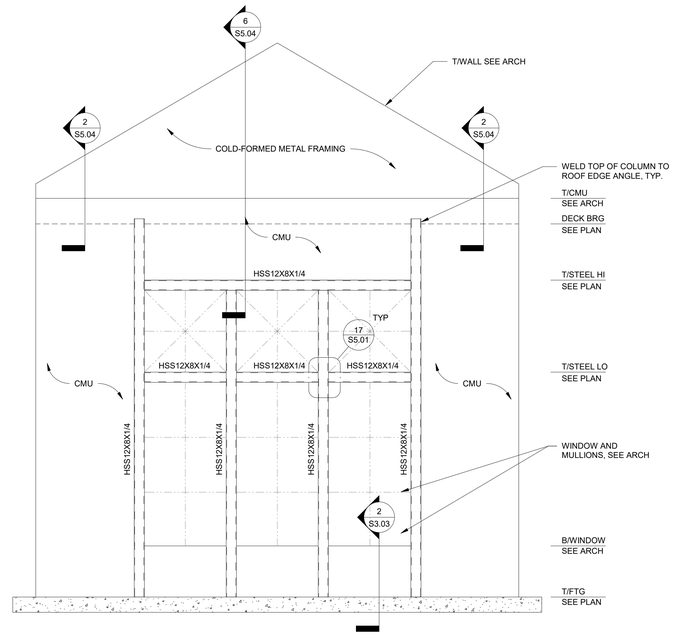
1 S5.04 SECTION THRU WALL ABOVE COVERED ENTRY, JOIST PERPENDICULAR
3/4" = 1'-0"



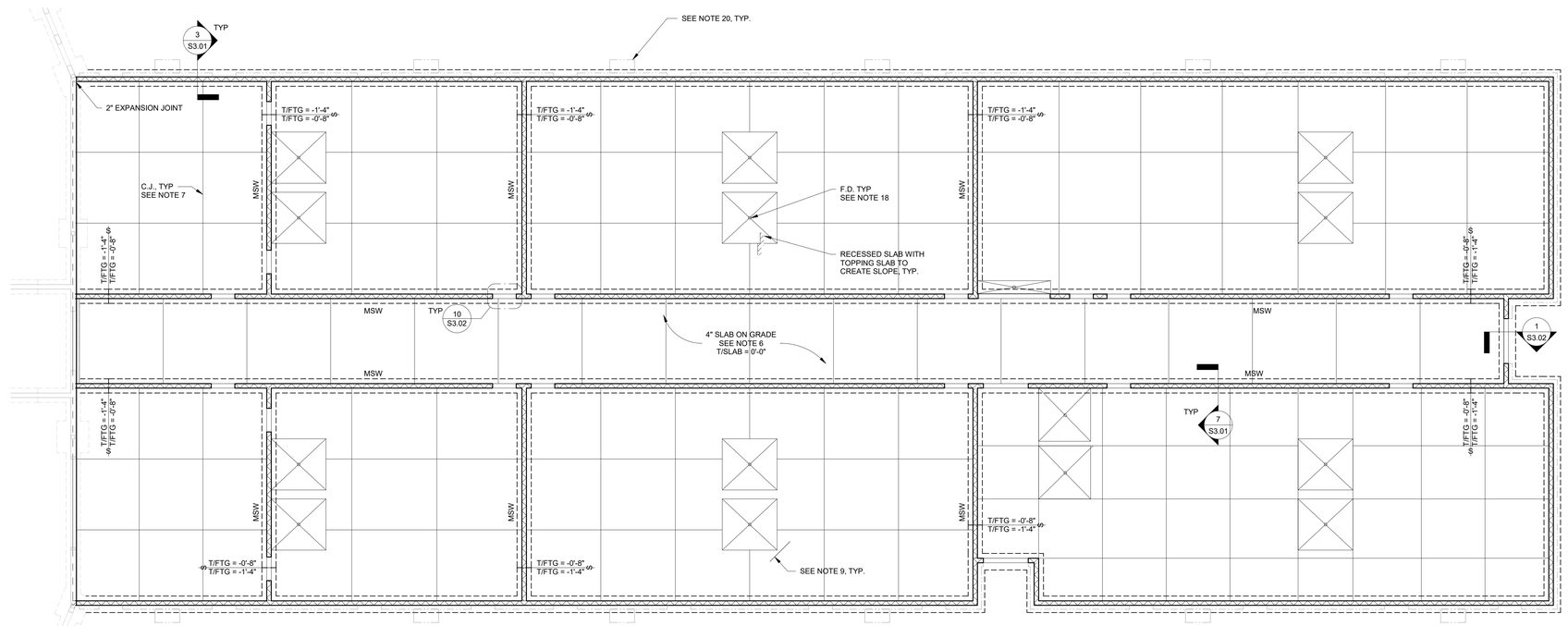
3 S5.10 FRAMING ELEVATION - CORNER OFFICE
1/4" = 1'-0"



13 S5.10 ELEVATION - FRONT ENTRANCE
1/4" = 1'-0"

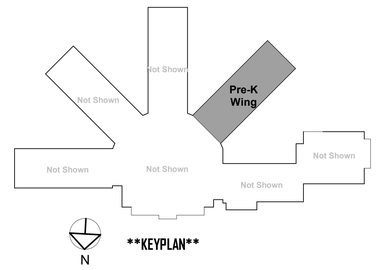


5 S5.10 FRAMING ELEVATION - CAFETERIA WINDOW
1/4" = 1'-0"



1 SA1.01 Additive Alternate "A" - Foundation Plan - Pre-K Wing
1/8" = 1'-0"

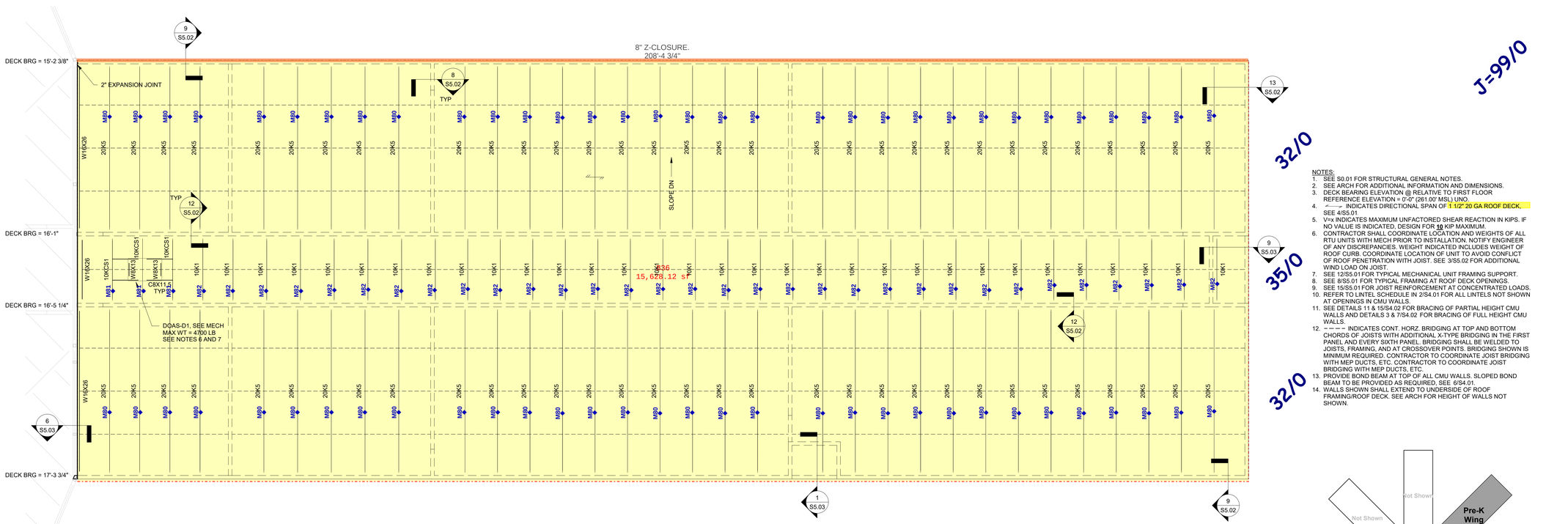
NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING. SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
8. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
9. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
10. INDICATES REINFORCED 8" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
11. INDICATES REINFORCED 12" MASONRY WALL. SEE WALL REINFORCING SCHEDULE BELOW AND DETAIL 1/S4.01.
12. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
14. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS. SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
15. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
16. SEE 10/S3.02. FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
17. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.
18. F.D. INDICATES FLOOR DRAIN. SLOPE SLAB TO DRAIN.
19. MSW INDICATES INTERIOR MASONRY SHEAR WALL. SEE DETAIL 17/S4.01. ALL EXTERIOR MASONRY WALLS SHOWN ARE SHEAR WALLS.
20. PROVIDE MASONRY AND BRICK LINTELS AT OPENINGS FOR WALL MOUNTED HEAT PUMPS. CONTRACTOR TO COORDINATE SIZE AND LOCATION WITH MECHANICAL. SEE 2/S4.01 FOR LINTEL SCHEDULE.
21. INDICATES SLAB DEPRESSION. SEE DETAIL 12/S3.02.



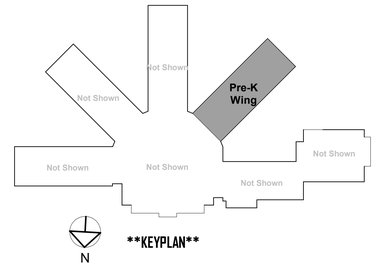
Deck Accessories (Single Page)					
	Label	Description	Total Length	Total Qtys	Comments-Add'l Loads
✓	8" Z-CLOSURE.	Other Accessories	208.40	210.00	Linear Feet, REF. 9/S5.02

Deck V4 (Single Page)										
	Description	Deck Type	Deck Gauge	Deck Finish	Lap or Waste	Deck SQ	No of Floors	Screw Pattern	Screw Total	Comments-Add'l Loads
✓	Roof	B36	20	G60	2%	159.41	1 Floor	12" O/C	5,314	.

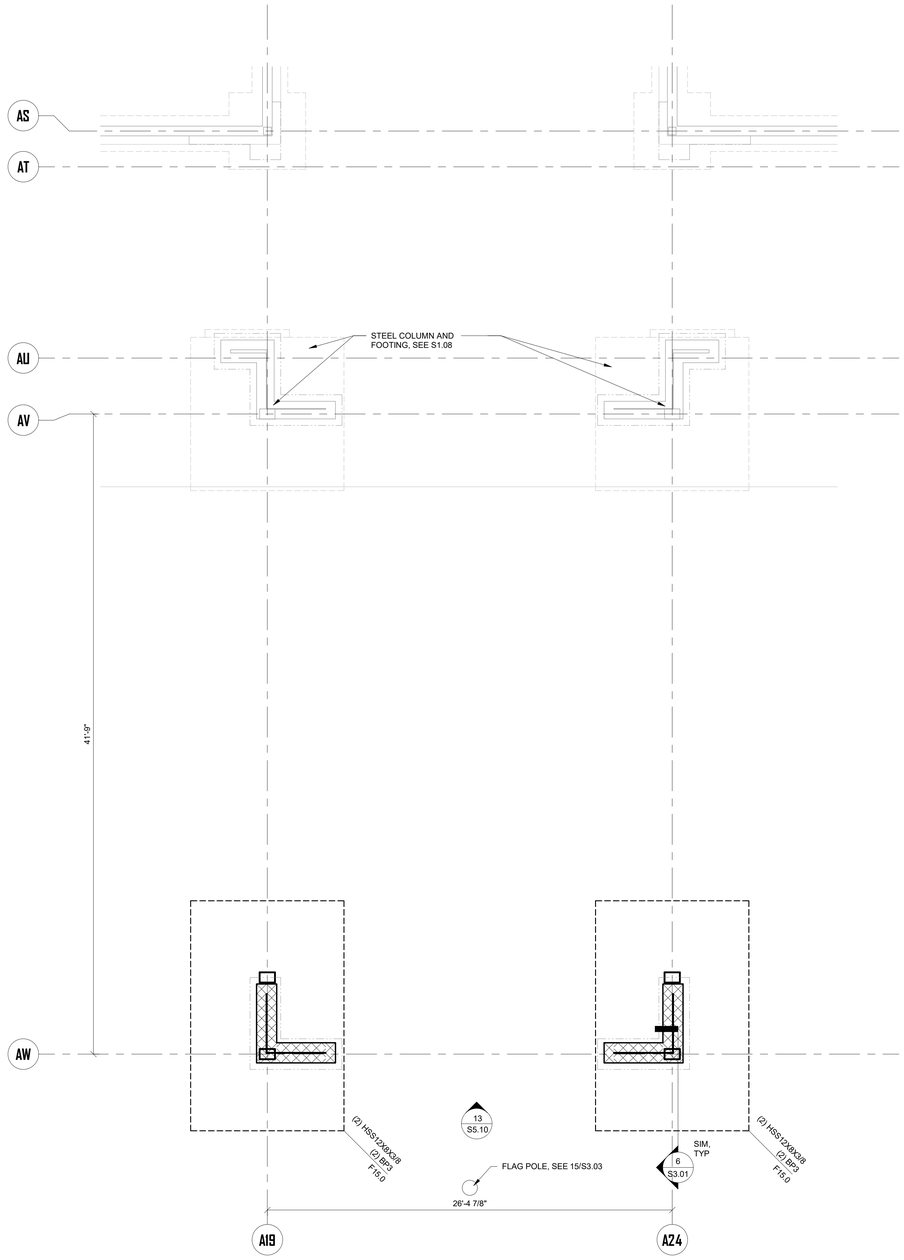
Joist	
Description	Quantity
M80	64
M81	3
M82	32



NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION @ RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. ←——→ INDICATES DIRECTIONAL SPAN OF 1 1/2" 20 GA ROOF DECK, SEE 4/S5.01
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR 10 KIP MAXIMUM.
6. CONTRACTOR SHALL COORDINATE LOCATION AND WEIGHTS OF ALL RTU UNITS WITH MECH PRIOR TO INSTALLATION. NOTIFY ENGINEER OF ANY DISCREPANCIES. WEIGHT INDICATED INCLUDES WEIGHT OF ROOF CURB. COORDINATE LOCATION OF UNIT TO AVOID CONFLICT OF ROOF PENETRATION WITH JOIST. SEE 3/S5.02 FOR ADDITIONAL WIND LOAD ON JOIST.
7. SEE 12/S5.01 FOR TYPICAL MECHANICAL UNIT FRAMING SUPPORT.
8. SEE 8/S5.01 FOR TYPICAL FRAMING AT ROOF DECK OPENINGS.
9. SEE 15/S5.01 FOR JOIST REINFORCEMENT AT CONCENTRATED LOADS.
10. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
11. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
12. – – – – INDICATES CONT. HORZ. BRIDGING AT TOP AND BOTTOM CHORDS OF JOISTS WITH ADDITIONAL X-TYPE BRIDGING IN THE FIRST PANEL AND EVERY SIXTH PANEL. BRIDGING SHALL BE WELDED TO JOISTS, FRAMING, AND AT CROSSOVER POINTS. BRIDGING SHOWN IS MINIMUM REQUIRED. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC. CONTRACTOR TO COORDINATE JOIST BRIDGING WITH MEP DUCTS, ETC.
13. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED, SEE 6/S4.01.
14. WALLS SHOWN SHALL EXTEND TO UNDERSIDE OF ROOF FRAMING/ROOF DECK. SEE ARCH FOR HEIGHT OF WALLS NOT SHOWN.

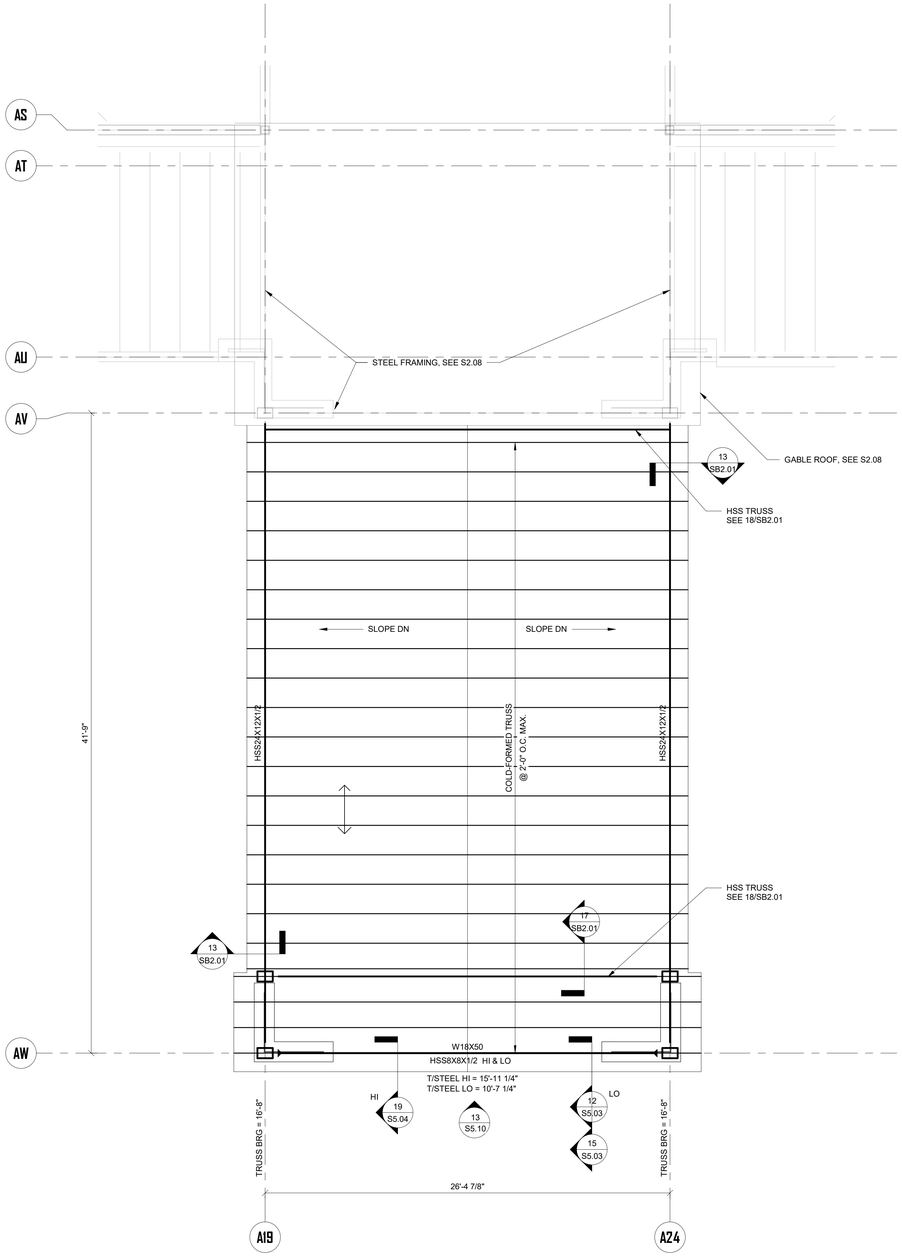


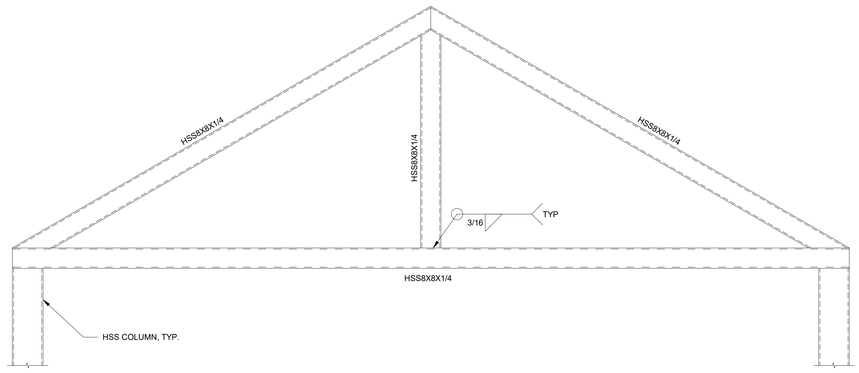
1 SA2.01 Additive Alternate "A" - Roof Framing Plan - Pre-K Wing
1/8" = 1'-0"



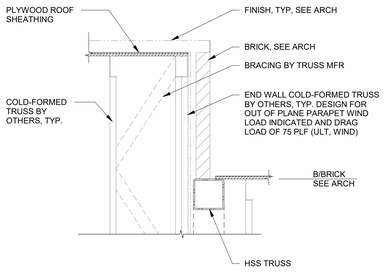
1 / S1.01 Additive Alternate "B" - Foundation Plan - Porte Cochere
1/4" = 1'-0"

NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. Fx INDICATES COLUMN FOOTING, SEE 1/S3.01. T/FTG = -1'-4" UNO, BASED ON T/SLAB REFERENCE ELEVATION = 0'-0" (261.00' MSL).
4. BPx INDICATES COLUMN BASE PLATE. SEE 1/S5.01.
5. INDICATES STEP IN FOOTING. SEE 17/S3.01.
6. PROVIDE 4" SLAB ON GRADE REINFORCED WITH WWF 6x6 W2.1xW2.1 ON VAPOR BARRIER AND 6" GRANULAR BASE.
7. C.J. INDICATES SLAB CONTROL JOINT. SEE 3/S3.02 AND GENERAL NOTES FOR ADDITIONAL INFORMATION.
8. PROVIDE ISOLATION JOINT AT COLUMN. SEE 11/S3.02.
9. PROVIDE REINFORCEMENT AT RE-ENTRANT CORNERS. SEE 7/S3.02.
10. INDICATES 8" MASONRY WALL REINFORCED W/ #5@32" O.C. SEE DETAIL 1/S4.01.
11. INDICATES 12" MASONRY WALL REINFORCED W/ #5@32" O.C. SEE DETAIL 1/S4.01.
12. SEE DETAILS 5/S4.01 AND 9/S4.01 FOR ADDITIONAL MASONRY REINFORCEMENT AT JAMBS, INTERSECTIONS, CONTROL JOINTS, CORNERS, AND JOIST/BEAM BEARINGS.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL UTILITY AND PLUMBING LINES. SEE 18/S3.01.
14. PROVIDE THICKENED SLAB AT ALL MASONRY PARTITION WALLS, SEE 2/S3.02 AND 6/S3.02. COORDINATE LOCATION OF MASONRY PARTITION WALLS WITH ARCH.
15. SEE 15/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DISCONTINUOUS CONTROL JOINTS.
16. SEE 10/S3.02 FOR ADDITIONAL SLAB REINFORCEMENT AT DOOR OPENINGS WITHIN LOAD-BEARING MASONRY WALLS.
17. SEE 4/S3.02 AND 8/S3.02 FOR PIPE/CONDUIT GROUPINGS WITHIN SLAB ON GRADE.

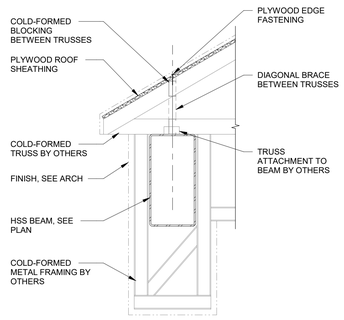




18 SB2.01 HSS TRUSS PROFILE
1/2" = 1'-0"



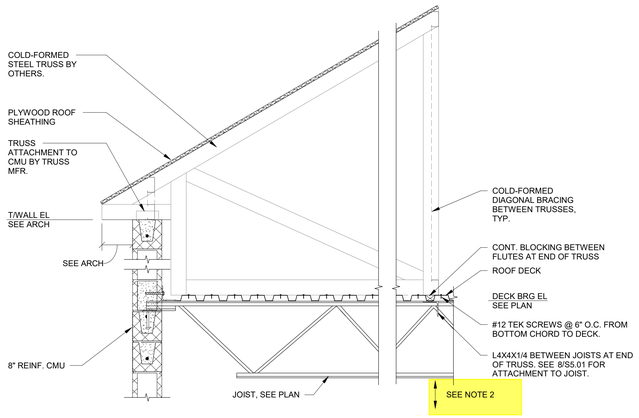
17 SB2.01 SECTION AT BRICK BEHIND PARAPET
3/4" = 1'-0"



13 SB2.01 SECTION AT PORTE COCHERE TRUSS BEARING
3/4" = 1'-0"

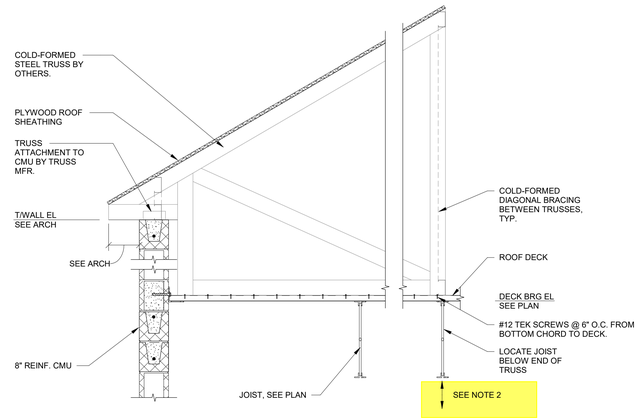
1 SB2.01 Additive Alternate "B" - Roof Framing Plan - Porte Cochere
1/4" = 1'-0"

NOTES:
1. SEE S0.01 FOR STRUCTURAL GENERAL NOTES.
2. SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
3. DECK BEARING ELEVATION RELATIVE TO FIRST FLOOR REFERENCE ELEVATION = 0'-0" (261.00' MSL) UNO.
4. <——> INDICATES DIRECTIONAL SPAN OF 3/4" FRT PLYWOOD SHEATHING, SEE 20/S5.04.
5. V=x INDICATES MAXIMUM UNFACTORED SHEAR REACTION IN KIPS. IF NO VALUE IS INDICATED, DESIGN FOR **15** KIP MAXIMUM.
6. ▶ INDICATES MOMENT CONNECTION. SEE 13 & 14/S5.01.
7. EXTEND AND ATTACH BOTTOM CHORD OF JOIST AT EACH COLUMN TO COLUMN, SEE DETAIL 7/S5.01.
8. REFER TO LINTEL SCHEDULE IN 2/S4.01 FOR ALL LINTELS NOT SHOWN AT OPENINGS IN CMU WALLS.
9. SEE DETAILS 11 & 15/S4.02 FOR BRACING OF PARTIAL HEIGHT CMU WALLS AND DETAILS 3 & 7/S4.02 FOR BRACING OF FULL HEIGHT CMU WALLS.
10. PROVIDE BOND BEAM AT TOP OF ALL CMU WALLS. SLOPED BOND BEAM TO BE PROVIDED AS REQUIRED, SEE 6/S4.01.
11. F=x INDICATES MAXIMUM ALLOWABLE AXIAL FORCE REQUIRED IN BEAM CONNECTION. STEEL FABRICATOR TO DESIGN CONNECTION FOR AXIAL FORCE INDICATED.



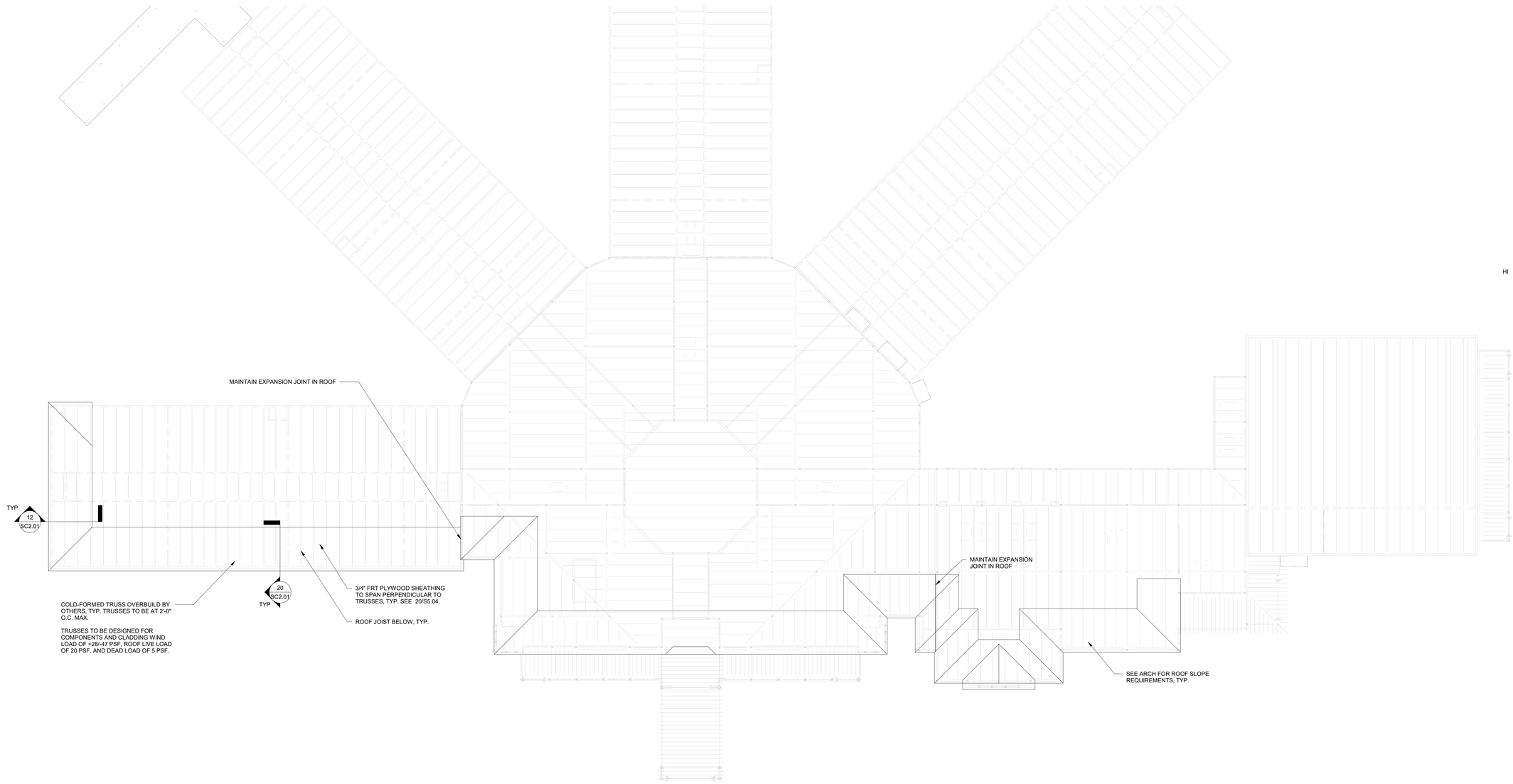
NOTES:
1. SEE 1/S5.02 FOR ADD'L. INFORMATION
2. JOIST TO BE DESIGNED FOR ADDITIONAL WIND LOAD OF 300 LB (ALLOWABLE).

20 / SC2.01 SECTION AT PITCHED ROOFS (JOIST PARALLEL TO TRUSS)
3/4" = 1'-0"



NOTES:
1. SEE 5/S5.02 FOR ADD'L. INFORMATION
2. JOIST TO BE DESIGNED FOR ADDITIONAL WIND LOAD OF 50 PLF (ALLOWABLE).

12 / SC2.01 SECTION AT PITCHED ROOFS (JOIST PERPENDICULAR TO TRUSS)
3/4" = 1'-0"



1 / SC2.01 Additive Alternate "C" - Roof Framing Plan - Pitched Roofs
1" = 20'-0"